

DATE: 17 July 2024 **CONFIDENTIALITY:** Confidential

SUBJECT: Cambushinnie 400kV Substation, 70108968

CLIENT: Balfour Beatty AUTHOR: Sophie Gedge

CHECKED: Darren Warnock APPROVED: Brennig Davis

INTRODUCTION

The proposed Cambushinnie 400 kV substation site is to be located approximately 4 km west of Braco, Perth and Kinross, Scotland, as shown in *Figure 1* below. The proposed site is located to the south of the existing Braco West Substation. The site is on the plateau of Feddal Hill and the surrounding land use is primarily commercial forestry plantations.

This Technical Note presents the findings of a review of available information including the Ground Investigation for the site commissioned by SSE. The aim of this note is to inform the expected ground conditions at the site, assess likely earthworks solutions and expected foundation solutions and discuss likely key geotechnical risks. It is the responsibility of the Client to make appropriate cost and contingency allowances in any tender submission to cover the geotechnical risks presented in this document, as well as qualifying and caveating their tender submission accordingly, based on the provisional review of geotechnical information in this document.



Figure 1 – Proposed Braco West Substation location (approximate)



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EARTHWORKS DESIGN REQUIREMENTS:

The proposed finished platform level is 241.0 m AOD. The site is currently sloping terrain and significant engineering earthworks are required.

All platform earthworks are to be undertaken in compliance with SP-NET-CIV-501 Specification for Earthworks Revision: 2.00 Date: February 2023.

HISTORICAL LAND USE

Historical mapping for the site has been obtained and extracts are included as **Appendix A**. The sequence of historical maps shows that the site remained undeveloped from 1866 through to 1979 as Heath/Rough Grassland/Bracken. The next available maps are 2001 and 2006 which show the land to be populated with coniferous trees and a few tracks. This is likely in relation to the Christmas tree farm present in the current day at the site and greater surrounding area. Mapping for 2023 shows the existing substation to the east and a few additional tracked roads to the west and south of the site.

PUBLISHED GEOLOGY

The following sources have been reviewed:

- BGS GeoIndex 1:50,000 mapping and borehole records
- BGS 1:50,000 geological map, Sheet 39W, Stirling, solid geology, 1976
- BGS 1:50,000 geological map, Sheet 39W, Stirling, drift, 1974
- BGS 1:10,560 geological map, NN70NE, solid and drift, 1961
- BGS 1:10,560 geological map, NN70NE, drift, 1982

The 1:50:000 BGS Online Geoindex map shows the superficial geology at the site to entirely comprise of *Peat* over *Till – Diamicton* present to the southern edge of the site boundary. The BGS lexicon of named rock units describes the Peat as *partially decomposed mass of semi-carbonized vegetation which has grown under waterlogged, anaerobic conditions, usually in bogs or swamps.* The Till - Diamicton is described as *heterogenous mixture of clay, sand, gravel, and boulders varying widely in size and shape.*

Glacial meltwater channels are shown within the Till, both to the north and south of the site and follow the generally topography of the land facing east towards Braco.

The BGS mapping information shows the bedrock geology to comprise Teith Sandstone Formation. This formation is described on the BGS 1:50,000 map as *purple, brown and grey sandstone with purple and brown siltstones and mudstones, also pebbly sandstones; passes laterally into conglomerate.* The older Cromlix Formation subcrops to adjacent to the east of the site. This formation is described on the BGS 1:50,000 map as *purple, red and brown mudstones and siltstones.* Both show to dip at 5 degrees in a general westerly direction. A fault is shown approximately 1 km to the south with a generally east to west orientation.



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AVAILABLE GROUND INVESTIGATION (GI) INFORMATION

2011 Ground Investigation for existing Braco substation.

For the existing Braco substitution, located to the northeast of the proposed new substation, SSEN commissioned a ground investigation that was designed by Waterman Transport and Development Ltd, with the fieldwork undertaken by Raeburn Drilling and Geotechnical Ltd (*Proposed Braco Substation Feddal Hill Report on Ground Investigation, 22305, November 2011*). The ground investigation comprised ten boreholes sunk by cable percussion and rotary open-hole and coring drilling methods to a maximum depth of 9m bgl. Eighteen trial pits were machine excavated to a maximum depth of 4m bgl, with termination depths varying based on localised ground conditions and excavation stability. The fieldwork was undertaken between 27th September and 5th October 2011.

A summary of the ground conditions recorded is presented in *Table 1* below.

Table 1 – Summary of encountered ground conditions (2011 Ground Investigation)

Strata - and typical description	Top Depth Range (m bgl)	Base Depth Range (m bgl)	Typical Thickness (m)
Peat – Dark brown partially rooty predominantly amorphous PEAT (H8/B3, H9/B3). Often with a surface layer, 0.05 to 0.3m thick, of moss and brown rooty PEAT (H3/B4, H4/B4) with tree stumps.	0.00	0.25 to 1.20	0.60 [Range 0.25 to 1.20]
Glacial Till – Predominantly cohesive, described as sandy / sandy gravelly CLAY with (occasional) cobbles. Material typically stronger with depth (soft becoming very stiff). Granular bands present typically described as very clayey very gravelly SAND. Upper 0.05m often described as a sandy gravelly SILT with relic rootlets.	0.25 to 1.20	1.70 to 5.00	2.30 [Range 0.50 to 4.50)
Bedrock – Teith Sandstone Formation: Predominantly Sandstone, weak to strong. Often with bands of extremely weak to weak Siltstone and Mudstone.	1.70 to 5.00	Not proven	Not proven



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INSITU TESTING

Standard Penetration Tests (SPTs) were undertaken at regular intervals through the superficial deposits. Hand vanes were completed for the trial pits within cohesive deposits and soils were sampled for laboratory testing.

GROUNDWATER

Water strikes were recorded in two boreholes (01, BHR02) at depths of 3.20 and 5.00m bgl and 0.0 to 0.8 m bgl respectively. Both are shown to be located on the northern side of the existing substation site. Four boreholes had standpipes with four rounds of weekly monitoring in October 2011, all results were recorded as dry. Gas monitoring was also undertaken and is summarised within the Factual Report.

SOIL CONTAMINATION

No Made Ground was observed in the ground investigation, and no asbestos was detected from the soil samples tested in the laboratory testing. This does not rule out the possibility of contaminated soil, given its location adjacent to the existing sub station and the historic and current land use as a Christmas tree farm (for example pesticides, oil/fuel leaks from machinery etc).

GEOTECHNICAL LABORATORY TESTING

Soil and rock samples were submitted for various geotechnical laboratory tests which are summarised as follows:

- 18 natural moisture content;
- 14 Atterberg tests;
- 11 particle size distribution tests (including sedimentation tests);
- 3 compaction tests (2.5kg);
- 4 moisture condition value (MCV) tests;
- 3 CBR tests;
- 4 oedometers;
- 7 undrained unconsolidated triaxial tests;
- 3 uniaxial compression tests;
- 14 point load tests; and
- 4 water content of rock tests.



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GEOENVIRONMENTAL LABORATORY TESTING

Soil samples were also submitted for various geoenvironmental laboratory tests which are summarised as follows:

- 7 metals, pH, cyanide, thiocyanate, total phenols, sulphide, total sulphur, total petroleum hydrocarbons, polyaromatic hydrocarbons (PAH) (USEPA 16);
- 7 BRE SD1 Suite D tests; and
- 7 asbestos identification tests;

2023 Ground Investigation (Site 2 and 3)

SSEN commissioned a ground investigation for the site area that was undertaken by IGNE (Raeburn) between 15th November and 8th December 2023. The investigation comprised fourteen boreholes and twenty trial pits. Boreholes were advanced by cable percussive, sonic and rotary core drilling methods to depths of 10 to 15 m. Trial pits were excavated by mechanical excavator to a maximum depth of 3 m. A total of 4600 Peat probes were also undertaken, across the site being taken forward (Site 2) and the adjacent area (Site 3). During the ground investigation it was discovered that Site 3 was underlain by a significant thickness of Peat and therefore planned exploratory holes were moved to Site 2 to improve coverage of the platform and laydown areas. An extract of the GI layout plan is indicated in *Figure 2* below:

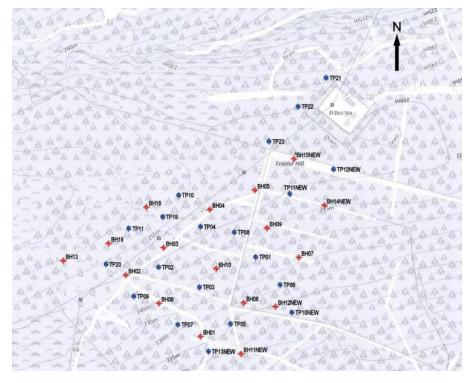


Figure 2 - GI Layout (2023)



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A summary of the ground conditions recorded is presented in Table 2 below.

Table 2 – Summary of encountered ground conditions (2023 Ground Investigation)

Strata - and typical description	Top Depth Range (m bgl)	Base Depth Range (m bgl)	Typical Thickness (m)
Peat – Soft brown to dark brown spongy pseudo-fibrous/ amorphous PEAT. Occasional pieces of wood.	0.00	0.20 to 3.00	0.80 [Range 0.20 to 3.00]
Glacial Till – Predominantly cohesive, described as reddish brown sandy gravelly CLAY with (occasional) cobbles. Material typically stronger with depth (soft to very stiff). Thick granular bands present and typically described as medium dense to very dense SAND or GRAVEL, ranging from very clayey to very sandy. Layer of SILT at interface with overlying PEAT.	0.20 to 3.00	0.90 to 5.50	1.60 [Range 0.40 to 4.30)
Bedrock – Predominantly SANDSTONE, weak to strong, with siltstone laminae. Factures are sub-horizontal and very closely to medium spaced. Occasional thick (>0.5 m) bands of weak Mudstone.	0.90 to 5.50	Not proven	Not proven

GROUND INVESTIGATION REPORT (GIR)

A Ground Investigation Report has been prepared by SLR Consulting Limited on behalf of SSEN (Braco West ASTI 400kV Substation, Ground Investigation Report, 21 May 2024). This report summarises in greater detail the same factual information obtained from the 2023 ground investigation.



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INSITU TESTING

Twenty Standard Penetration Tests (SPTs) were undertaken at regular intervals through the superficial deposits. Hand vanes were completed for the trial pits within cohesive deposits and soils were sampled for laboratory testing.

Nine Dynamic Cone Penetrometer (DCP) tests were undertaken adjacent to ten of the trial pits, with results providing cumulative blow count and correlated California Bearing Ratio with depth.

10Tthermal resistivity tests were undertaken

Six soakaway tests were undertaken in four trial pits which are summarised in Table 3 below.

Table 3 – Summary of soakaway infiltration tests

Test Location	Ground conditions at base of test pit	Infiltration Rate	Comments
TP03	Reddish brown silty SAND and GRAVEL with cobbles.	Indeterminate	Test terminated due to slow water outflow (drainage).
TP06 (1)	Greyish brown SANDSTONE.	1.1 E-03 m/s	Unable to fill pit above 1.1 m due to filtration rate.
TP06 (2)	Greyish brown SANDSTONE.	3.6 E-04 m/s	Unable to fill pit above 1.1 m due to filtration rate.
TP06 (3)	Greyish brown SANDSTONE.	4.2 E-04 m/s	
TP07	Greyish brown slightly clayey slight gravelling SANDSTONE with cobbles and boulders	Indeterminate	Test terminated due to slow water outflow (drainage).
TP13	Reddish brown very gravelly clayed SAND with cobbles	Indeterminate	Test terminated due to slow water outflow (drainage).



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GROUNDWATER

No water strikes were recorded in boreholes of the 2023 ground investigation, possibly due to the method of drilling. Seepages were recorded in trial pits TP07 and TP09 at 1.2 mbgl and 2.0 mbgl. Standpipes were installed in nine of the boreholes and four rounds of monitoring return visits were undertaken. The monitoring recorded that four of the installations (boreholes BH04, BH10, BH13 and BH14) remained dry throughout. The remaining installations recorded groundwater levels between 0.88 mbgl and 8.77 mbgl, as shown in Table 4. Gas monitoring was also undertaken and is summarised within the Factual Report.

Table 4 – Summary of groundwater monitoring

Wellpoint ID	Depth of Installation (mbgl)	Round 1 Water Level (mbgl)	Round 2 Water Level (mbgl)	Round 3 Water Level (mbgl)	Round 4 Water Level (mbgl)
BH01	5.00	2.44	2.44	2.42	3.95
BH02	8.00	4.78	4.88	4.91	4.9
BH04	2.00	dry	dry	dry	dry
BH07	10.00	4.51	4.62	4.67	7.12
BH10	1.50	dry	dry	dry	dry
BH11	5.00	1.02	0.88	0.91	1.48
BH13	2.80	dry	dry	dry	dry
BH14	2.70	dry	dry	dry	dry
BH19	10.00	8.74	8.73	8.77	dry



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GEOTECHNICAL AND GEOENVIRONMENTAL LABORATORY TESTING

Soil and rock samples were submitted for various geotechnical laboratory tests for classification and determination of strength parameters, and also geoenvironmental testing. The tests undertaken are summarised below as Table 5.

Table 5 – Summary of laboratory tests (duplicated from GIR (SLR, 2024)).

Laboratory Test	Number of Tests
Soil Testing	
Moisture content	69
Atterberg Limits	13
Particle size distribution by wet sieving	50
Sedimentation by pipette	44
Dry density/moisture content relationship using 2.5 kg rammer	2
Dry density/moisture content relationship using 4.5 kg rammer	7
Moisture Condition Value at natural moisture content	8
California Bearing Ratio on re-compacted disturbed sample	5
Suite A (Greenfield site – pyrite absent Schedule 1.19.6)	30
Rock Testing	
Single measurement of point load strength on irregular rock lump or core sample (either axial or diametral test)	340
Uniaxial compressive strength	37
LA Coefficient	7
Environmental Testing	
Suite E (Soil samples Schedule S1.20.3)	19
Suite H (Inert waste landfill Schedule S1.20.5)	1



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GENERALISED GROUND MODEL

The generalised ground model presented in Table 6 is based on the results of the 2023 ground investigation which was undertaken on the proposed site. It is noted that this ground model may not be representative of ground conditions below the entire site as variation in strata thicknesses is likely at a local level. Earthworks volumes are based on triangulation of exploratory hole locations and interpolating layer thicknesses between those locations. The ground model derived by SLR and presented within the GIR is largely in agreement with the table below.

Table 6 - Generalised Ground Model

Strata and typical description	Top Depth (m bgl)	Base Depth (m bgl)	Typical thickness (m)
Peat - Soft brown to dark brown spongy pseudo-fibrous/ amorphous PEAT	0.00	0.80	0.80
Superficial Deposits: Glacial Till - Predominantly cohesive, described as sandy / sandy gravelly CLAY with (occasional) cobbles. Material typically stronger with depth (soft becoming very stiff).	0.80	2.40	1.60
Thick granular bands present typically described as very clayey very gravelly SAND or GRAVEL.			
Bedrock : Teith Sandstone Formation : Predominantly Sandstone, weak to strong. Often with bands of extremely weak to weak Siltstone and Mudstone. Upper layer highly / completed weathered, recovered as gravel.	2.40	Not proven	Not Proven

Assumed groundwater level is 2 m below existing ground level, based on four rounds of monitoring of groundwater installations. Note that groundwater is expected to vary and may be locally higher.



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EARTHWORKS STRATEGY

Ground levels at the site vary between approximately 253 m AOD at the northern corner and 230 m AOD in the southern corner. The proposed finished platform level is 241 m AOD however due to the sloping nature of the site, sloping downward from north to south, significant cut and fill will be required to create a level platform (predominantly fill). The level of the platform is governed by acceptable access road profiles. Maximum fill depth, above existing ground level (and excluding any dig and replace of unsuitable material) is approximately 14 m at the southern edge of the platform. A cutting slope up to approximately 9 m deep is required along the northern platform edge.

Below proposed fill areas, any variable, soft or compressible soils (including Topsoil, Peat and any soft Glacial Till where present) will require removal and replacement with suitable engineered fill prior to upfilling.

Preliminary earthworks drawings are included in **Appendix B** and provisional quantities for the 400 kV platform, based on the 2023 ground investigation, are currently estimated to be as follows:

- Cut arisings (from cut areas and removal of unsuitable soil from formation prior to placement in fill areas): 289,095 m³ (of which 67,578 m³ is Topsoil / Peat, 71,233 m³ is Glacial Till or weathered rock, and 150,284 m³ is rock).
- Engineered fill requirement: 291,663 m³ (engineered fill from formation, substation drainage layers and platform surface finishes).

Provisional quantities for the access roads, based on the 2023 ground investigation, are currently estimated to be as follows:

- Cut arisings (from cut areas and removal of unsuitable soil from formation prior to placement in fill areas): 41,774 m³ (of which 33,916 m³ is Topsoil / Peat, 6,235 m³ is Glacial Till or weathered rock, and 1,623 m³ is rock).
- Engineered fill requirement: 43,060 m³ (engineered fill from formation to finished road level)

Provisional quantities for the drainage basin are currently estimated to be as follows:

- Cut arisings: 807 m³.
- Engineered fill requirement: 2,548 m³ (to form basin perimeter bund).

It is considered that the Topsoil and Peat are unsuitable for re-use as an engineered fill and will require disposal where these are present within the footprint of the platform earthworks. The superficial deposits, considered to be both Glacial Till and the weathered rock profile, are likely to be a mixed soil comprising cohesive and granular elements. Where primarily granular, superficial deposits may be suitable for reuse as engineered fill, subject to some careful selection and correct handling. Primarily cohesive Glacial Till is unlikely to be suitable for reuse without modification (e.g., lime, cement). The underlying competent



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sandstone rock, where locally exposed in cut areas, should also be suitable for crushing, processing and re-use as engineered fill.

Indicatively, assuming that 50% of the Glacial Till is reused as general fill, there is a general <u>surplus</u> of <u>unsuitable</u> soils (approximately 141,035 m³) and a <u>shortfall</u> of engineered fill required (approximately 146,630 m³). These figures do not include for any bulking or compaction related changed in volume. Unsuitable soils comprise Topsoil and Peat and any soft cohesive Glacial Till where present. These figures are subject to change depending on Contractor preference to process site won superficial materials or import granular fill. Should all of the Glacial Till be deemed not suitable for processing for use as general upfill, the estimated shortfall of engineered fill would increase to approximately 185,364 m³.

Excavations into the superficial deposits at the site may be unstable as perched groundwater may be present at shallow depths within granular layers. Excavations would therefore need to be supported or battered back to safe angles. Groundwater management using sump pumping is likely to be sufficient for any dewatering necessary during construction as part of the proposed developments – subject to confirmation of groundwater regime. Once constructed, the engineered fill wedge will be generally free draining and existing groundwater flows will re-establish within and below the platform and in the underlying superficial deposits.

CUT SLOPES

A cut slope is required at the northern edge of the platform where existing ground levels are up to approximately 9 m higher than the proposed platform level.

Based on the generalised ground model, these cut slopes will be within Peat, Glacial Till and Sandstone Rock. As the proposed cut is likely to extend into bedrock the use of rock breakers may be required, which may be extensive should strong rock be encountered in any cut areas or for foundation, drainage or ducting excavations. It is not expected that blasting would be required as the bedrock is sedimentary which can generally be easily ripped.

At this stage it is anticipated that a conventional cut slope will be possible without requirement for strengthening measures.

For cut slopes in Glacial Till it is likely that a slope of approximately 1v:3h (18.4°) will be suitable. The gradient of any cut slope through Peat should be slackened to a maximum gradient of 1v:4h (14°) or shallower, or locally removed and replaced with granular fill if / where exposed in any cut faces. If / where rock is exposed in cut slopes, this can be cut at 1v:1h (45°), or potentially steeper subject to detailed stability analysis.



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FILL SLOPES

Where fill slopes are required these shall be formed from either site won or imported granular engineered fill. Depending on any imposed loads or groundwater conditions, the fill slopes may be designed to gradients of 1v:3h (18.4°) where formed of granular material. It is not recommended that the fill slopes are formed using cohesive material.

At this stage it is anticipated that conventional earthworks fill slopes will be possible without requirement for strengthening measures (e.g., reinforced soil, soil nailing).

MATERIAL RE-USE

A material re-use assessment has been undertaken based on results of the 2023 ground investigation. A summary of the assessment is as follows:

- The Topsoil Peat or any soft cohesive Glacial Till (where present) will not be suitable for use as engineered fill but may be suitable for use as landscaped fill or peat restoration.
- The majority of the site-won superficial deposits are likely to classify as a Class 1A or 2C material, with an assumption that 50% of this is suitable for re-use as general upfill earthworks, with processing to remove oversized particles or soft or low strength material.
- A review of the oversize content within the superficial deposits suggests that is may be possible to bulk crush and process the oversized content to produce a Class 1 fill material suitable for use as bulk upfill earthworks. The fines content of the natural soils within the proposed area of cut is approximately 17%.
- Any rock excavated/recovered is expected to be predominately red Sandstone (non-argillaceous).
 This can be bulk crushed and processed into structural fill (Class 1 and/or Class 6 subject to additional testing).
- Discrete layers of siltstone or mudstone, of varying thickness, are present below the site and these materials are argillaceous and may not be reused in platform construction layers 1-3.

PLATFORM MAKE UP

The make-up of the platform fill and surfacing shall be in accordance with SSEN Specification for Earthworks (SP-NET-CIV-501, rev 2.0), as shown below as Figure 3.

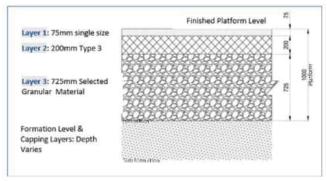


Figure 2 – Substation platform make up (Figure 5.1 of SP-NET-CIV-501 rev 2.0)



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Additionally, the SSEN Specification for Earthworks clarifies as follows:

- Layer 1: Single size durable aggregate is to be used as a surfacing material out with areas of
 concrete and asphalt bound aggregate surfaces. This layer is required to mitigate step and touch
 electrical earthing potentials and shall be a minimum of 75mm thick with 20mm single-sized
 washed aggregate. This layer is required to be free draining.
- Layer 2: The layer beneath the single size aggregate shall consist of 200mm Type 3 granular material (as defined in Series 800 of the SHW). This layer is required to be free draining.
- Layer 3: This shall consist of a minimum of 725mm well graded selected granular material. Class 6F2 (as defined in Series 600 of the SHW) is pre-approved by The Employer for this layer (beneath the 200mm Type 3). This layer shall be required to be free draining.

EXPECTED FOUNDATION SOLUTIONS

Based on the generalised ground model for the site and on the assumption that Peat and any soft Glacial Till below the platform is fully excavated and replaced, it is considered that conventional ground bearing shallow foundations are likely to be suitable at the site, bearing within either placed engineered granular fill or the existing firm or stiff Glacial Till, weathered rock or bedrock. The allowable bearing capacity of placed granular fill is anticipated to be 150 kN/m², which is the minimum requirement for the sub-formation as specified within SSEN Specification for Earthworks (SP-NET-CIV-501, rev 2.0)

If different material types are expected across the foundation footprint (i.e., fill / superficial soil / rock), these should be locally deepened to bear on similar material. Specific combinations of applied loads and ground conditions may require deeper pad foundations or piling to limit settlement. This will be confirmed during detailed design.

SOIL CONTAMINATION AND WASTE DISPOSAL

A full assessment of soil contamination risks is outside the scope of this report. It is understood that the recent SSEN ground investigation will be reviewed by contaminated land specialists once the information has been made available.

Soils requiring removal from site as either unsuitable due to contamination status or geotechnically unsuitable (such as the Peat and some of the Made Ground) are not expected to be classified as *Inert* if disposed to landfill and may be classified as either *Non-hazardous* or *Hazardous* waste. Further soils analysis including Waste Acceptance Classification (WAC) tests will be required one the relevant soils and disposal faciality have been identified. All excavated soils must be carefully segregated to enable selective disposal of different waste classifications as required.



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UNEXPLODED ORDNANCE

SSEN commissioned a UXO desk study and risk assessment that was prepared by Zetica UXO (Ref: P13558-23-R1). The risk assessment for the considered site indicates that the UXO risk is low for encountering unexploded bombs or other unexploded ordnance.

Table 3 – UXO Risk (Zetica UXO Desk Study and Risk assessment (P13558-23-R1)

Potential UXO Hazard	Anticipated Works	PE	PD	P = PE x PD	Likelihood	Severity	Risk Rating	UXO Risk
	Excavations	1	1	1	1	5	5	Low
UXB	Peat Probing	1	1	1	1	5	5	Low
	Boreholes	1	1	1	1	4	4	Low
	Shallow Excavations	1	1	1	1	4	4	Low
Other UXO	Peat Probing	1	1	1	1	4	4	Low
	Boreholes	1	1	1	1	3	3	Low

PEAT

The presence and thickness of Peat varies across the site. If left in situ this would give rise to very high total and differential settlements below the platform, substation surfaced areas and foundations and should therefore be removed. For the purposes of the tender design it is assumed that all Peat shall be excavated and removed from below the footprint of the platform and access road and, in fill areas, replaced with site won or imported granular engineered fill.



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KEY GEOTECHNICAL RISKS AND ASSUMPTIONS

The key geotechnical risks considered relevant to the scheme based on the information reviewed are listed below, and are discussed in detail in the *Geotechnical Risk Register* presented in **Appendix C**. Geotechnical assumptions are also presented in a *Geotechnical Assumptions Register* in **Appendix C**.

- Potential for soft or loose ground or Peat (and/or potentially with greater thickness than
 encountered during the GI) leading to low and variable bearing capacities and/or high total or
 differential settlement foundations may need to be deepened to reach more competent strata and
 more unsuitable material may need to be locally removed.
- Localised high groundwater levels leading to instability of excavations or slopes. Groundwater
 monitoring shows near surface groundwater levels in two boreholes and two trial pits encountered
 groundwater within 2 m of ground level. Localised potential for 'running sand' conditions to
 temporarily develop during earthworks if surface and groundwater not controlled.
- Potential for rockhead being higher than anticipated in the areas of the proposed cut slope, resulting in more of the cut and slopes being in rock, increasing excavation difficulty and likely extending programme.

SUPPLEMENTARY GROUND INVESTIGATION FOR DETAILED DESIGN

The ground investigation undertaken generally provides reasonable coverage of the site however there are notable gaps in the data required to complete the detailed design where existing exploratory holes are widely spaced.

A supplementary ground investigation scope has been prepared and issued under separate cover.

MATERIAL MANAGEMENT PLAN

A Material Management Plan (MMP) prepared in line with CL:AIRE voluntary code of conduct is likely to be required to satisfy waste legislation to cover the re-use of excavated surplus soils from the site (on this site or another site) which may otherwise be considered as waste soils. This will require review of the proposed material movements through the life cycle of construction, validation by the Qualified Person (QP), registration of the scheme with the Scottish Environmental Protection Agency (SEPA) and ultimate sign off on completion. Note that this does not cover material movements associated with any remediation, if required, or off-site disposal to landfill.



DATE: 17 July 2024 **CONFIDENTIALITY:** Confidential

SUBJECT: Cambushinnie 400kV Substation, 70108968

CLIENT: Balfour Beatty AUTHOR: Sophie Gedge

CHECKED: Darren Warnock APPROVED: Brennig Davis

APPENDIX A
HISTORICAL MAPS



Envirocheck® Report:

Historical Data Report Datasheet

Order Details:

Order Number:

327698237_1_1

Customer Reference:

70108968

National Grid Reference:

279170, 709030

Slice:

Α

Site Area (Ha):

14.26

Search Buffer (m):

1000

Site Details:

Site at

Braco

Perth and Kinross

Client Details:

Ms S Gedge WSP UK Ltd 110 Queen Street Glasgow Lanarkshire G1 3BX







Report Section	Page Number
Summary	-
Historical Building Plans Information	-
Historical Land Use Information	1
Historical Tanks and Energy Facilities	-
Historical Map List	2
Useful Contacts and Further Information	3

Introduction

The Environment Act 1995 has made site sensitivity a key issue, as the legislation pays as much attention to the pathways by which contamination could spread, and to the vulnerable targets of contamination, as it does the potential sources of contamination. For this reason, Landmark's Site Sensitivity maps and Datasheet(s) place great emphasis on statutory data provided by the Environment Agency and the Scottish Environment Protection Agency; it also incorporates data from Natural England (and the Scottish and Welsh equivalents) and Local Authorities; and highlights hydrogeological features required by environmental and geotechnical consultants. It does not include any information concerning past uses of land. The datasheet is produced by querying the Landmark database to a distance defined by the client from a site boundary provided by the client.

In the attached datasheet the National Grid References (NGRs) are rounded to the nearest 10m in accordance with Landmark's agreements with a number of Data Suppliers.

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Report Version v53.0



Summary

Data Type	Page Number	On Site	0 to 250m	251 to 500m	501 to 1000m
Historical Building Plans Information					
Areas Cleared Due To Enemy Action					
Above Ground Fuel Tanks (100m)				n/a	n/a
Asbestos (100m)				n/a	n/a
Benzene/Benzole/Naphtha, Naphthalene/Kerosene (100m)				n/a	n/a
Electricity Generation (100m)				n/a	n/a
Electricity Sub-Stations (100m)				n/a	n/a
Gas Industry (100m)				n/a	n/a
Gas Storage (100m)				n/a	n/a
Gas Use (100m)				n/a	n/a
Oil Industry (100m)				n/a	n/a
Oil Storage (100m)				n/a	n/a
Oil Use (100m)				n/a	n/a
Paint based Oils (100m)				n/a	n/a
Paraffin (100m)				n/a	n/a
Petrol and Diesel Industry (100m)				n/a	n/a
Petrol and Diesel Storage (100m)				n/a	n/a
Petrol and Diesel Use (100m)				n/a	n/a
Potential Fuel Gas (100m)				n/a	n/a
Potential Fuel Oil (100m)				n/a	n/a
Potential Fuel Use (100m)				n/a	n/a
Potential Petrol and Diesel (100m)				n/a	n/a
Potential Tanks (100m)				n/a	n/a
Potentially Fuel-related Tanks (100m)				n/a	n/a
Underground Fuel Tanks (100m)				n/a	n/a
Historical Land Use Information					
Former Marshes					
Historical Flood Liabilities					
Potentially Contaminative Industrial Uses (Past Land Use)					
Potentially Infilled Land (Non-Water)					
Potentially Infilled Land (Water)	pg 1				1



Summary

Data Type	Page Number	On Site	0 to 250m	251 to 500m	501 to 1000m
Historical Tanks and Energy Facilities					
Electrical Sub Station Facilities (100m)				n/a	n/a
Electricity Industry Facilities (100m)				n/a	n/a
Gas Industry Facilities (100m)				n/a	n/a
Gas Monitoring Facilities (100m)				n/a	n/a
Miscellaneous Power Facilities (100m)				n/a	n/a
Oil Industry Facilities (100m)				n/a	n/a
Petroleum Storage Facilities (100m)				n/a	n/a
Potential Tanks (100m)				n/a	n/a
Tanks (100m)				n/a	n/a



Historical Land Use Information

Map ID		Details	Quadrant Reference (Compass Direction)	Estimated Distance From Site	Contact	NGR
	Potentially Infilled	Land (Water)				
1	Use: Date of Mapping:	Unknown Filled Ground (Pond, marsh, river, stream, dock etc) 1901	A7NE (E)	527	1	279832 708777

Order Number: 327698237_1_1 Date: 07-Dec-2023 rpr_ec_datasheet v53.0 A Landmark Information Group Service Page 1 of 3





No Historical Building Plans information available.

The following mapping has been analysed for Historical Land Use Information:

1:10,560	Mapsheet	Published Date
Perthshire	116_00	1866
Perthshire	116_SE	1901
Ordnance Survey Plan	NN70NE	1958
Ordnance Survey Plan	NN80NW	1958
Ordnance Survey Plan	NN81SW	1959
1:10,000	Mapsheet	Published Date
Ordnance Survey Plan	NN70NE	1978
Ordnance Survey Plan	NN71SE	1978
Ordnance Survey Plan	NN80NW	1979
Ordnance Survey Plan	NN81SW	1982

No Historical Tanks and Energy Facilities information available.

Order Number: 327698237_1_1 Date: 07-Dec-2023 rpr_ec_datasheet v53.0 A Landmark Information Group Service Page 2 of 3



Useful Contacts and Further Information

Contact	Name and Address	Contact Details
1	Landmark Information Group Limited Imperium, Imperial Way, Reading, Berkshire, RG2 0TD	Telephone: 0844 844 9966 Fax: 0844 844 9951 Email: helpdesk@landmark.co.uk Website: www.landmark.co.uk

Historical Building Plans Information

This data set contains potentially contaminative features such as asbestos, petrol, oil and tanks captured from Historical Building Plans. The Historical Building Plans were produced by the London-based firm Charles E. Goad Ltd. as fire insurance plans, dating back to 1885. The firm ceased production of fire insurance plans in 1970. Most of the important towns and cities of the British Isles are covered. Historical Building Plans are usually at the scales of 1:480 (1 inch to 40 feet) for the British Isles. They were updated every 5-6 years by means of revision sheets designed to be pasted on to the original plans.

It should be noted that Historical Building Plans are only available for certain major towns and cities and in some cases there may only be partial coverage of the search area. It cannot therefore be assumed that the absence of responses under the Historical Building Plans section of this report indicates that no hazards exist. Please check the Historical Building Plans Map List table in the Historical Map List section of this report to establish if Historical Building Plans are available for this search area.

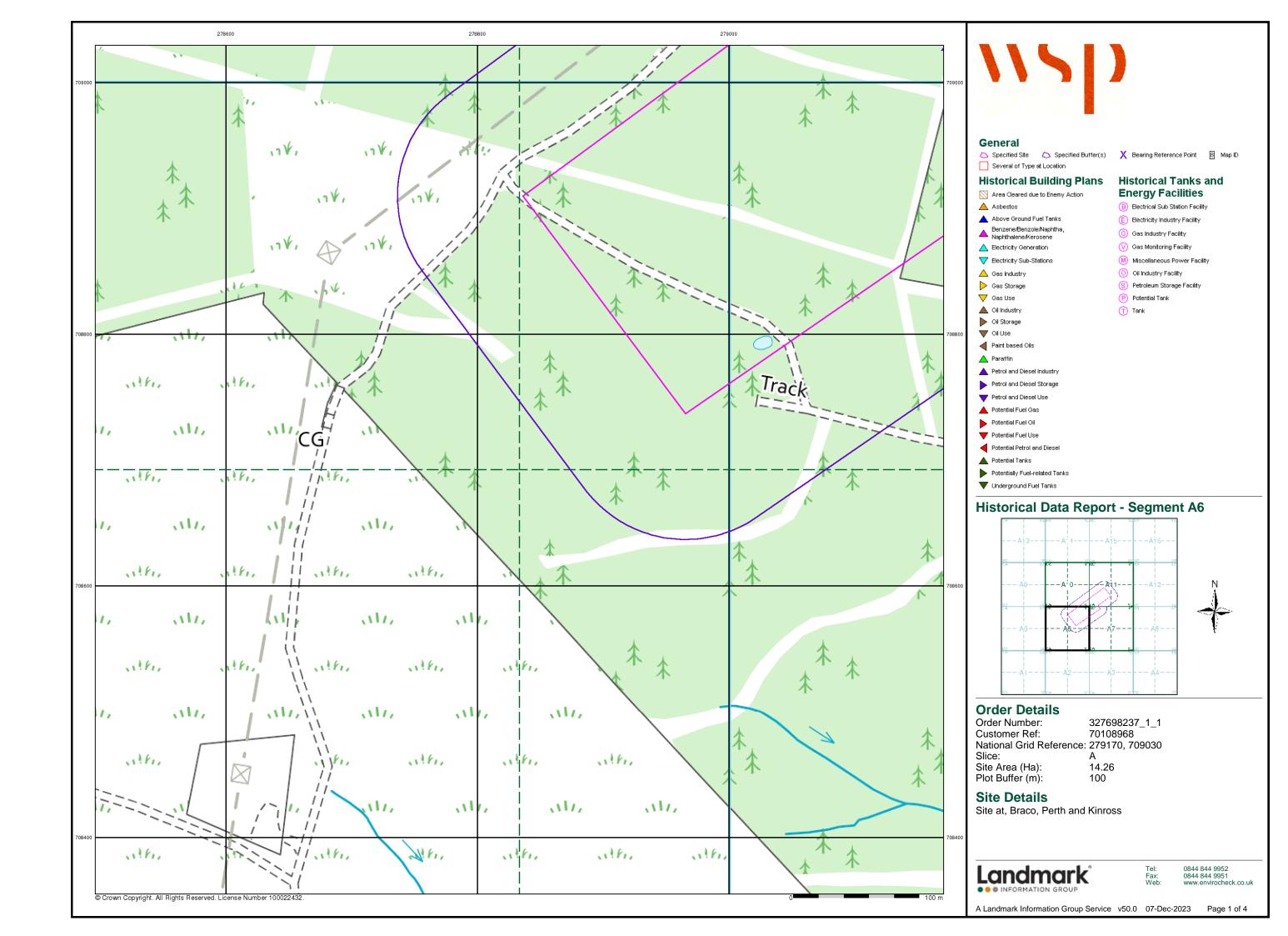
Historical Land Use Information

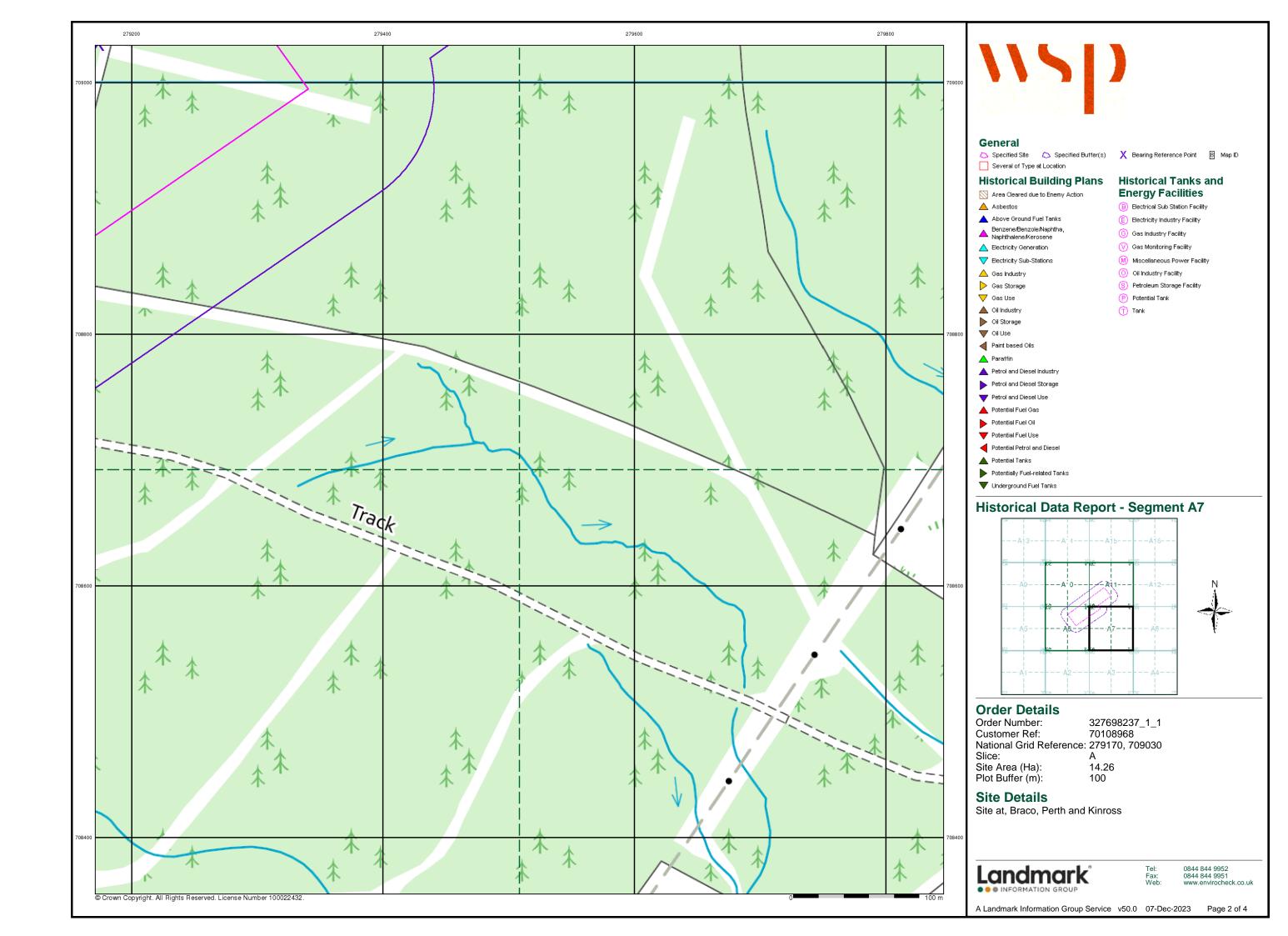
Landmark's Historical Land Use Data is the result of combined analysis of historical map data captured at 1:10,560 and 1:10,000. A unique comprehensive database of Historic Land Use from the 1840's to 1996 it includes 67 different types of potentially contaminated past industrial land use. This entailed analysing over 60,000 maps and is drawn from at least four, and up to six historical map editions. In addition a seventh layer was also created, known as the land use layer, containing areas of infilled land which are plotted via comparison between two or more map editions.

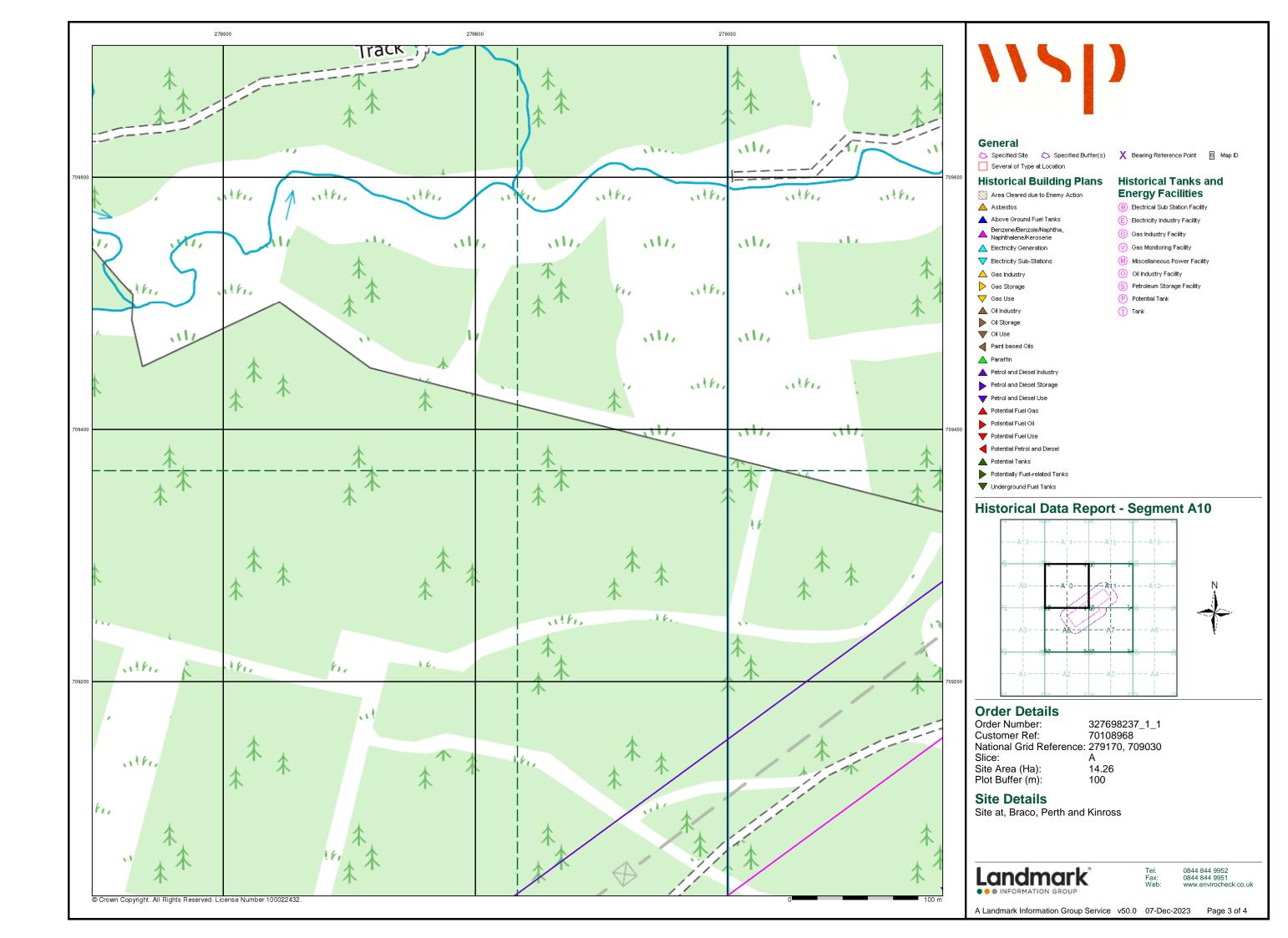
Historical Tanks and Energy Facilities

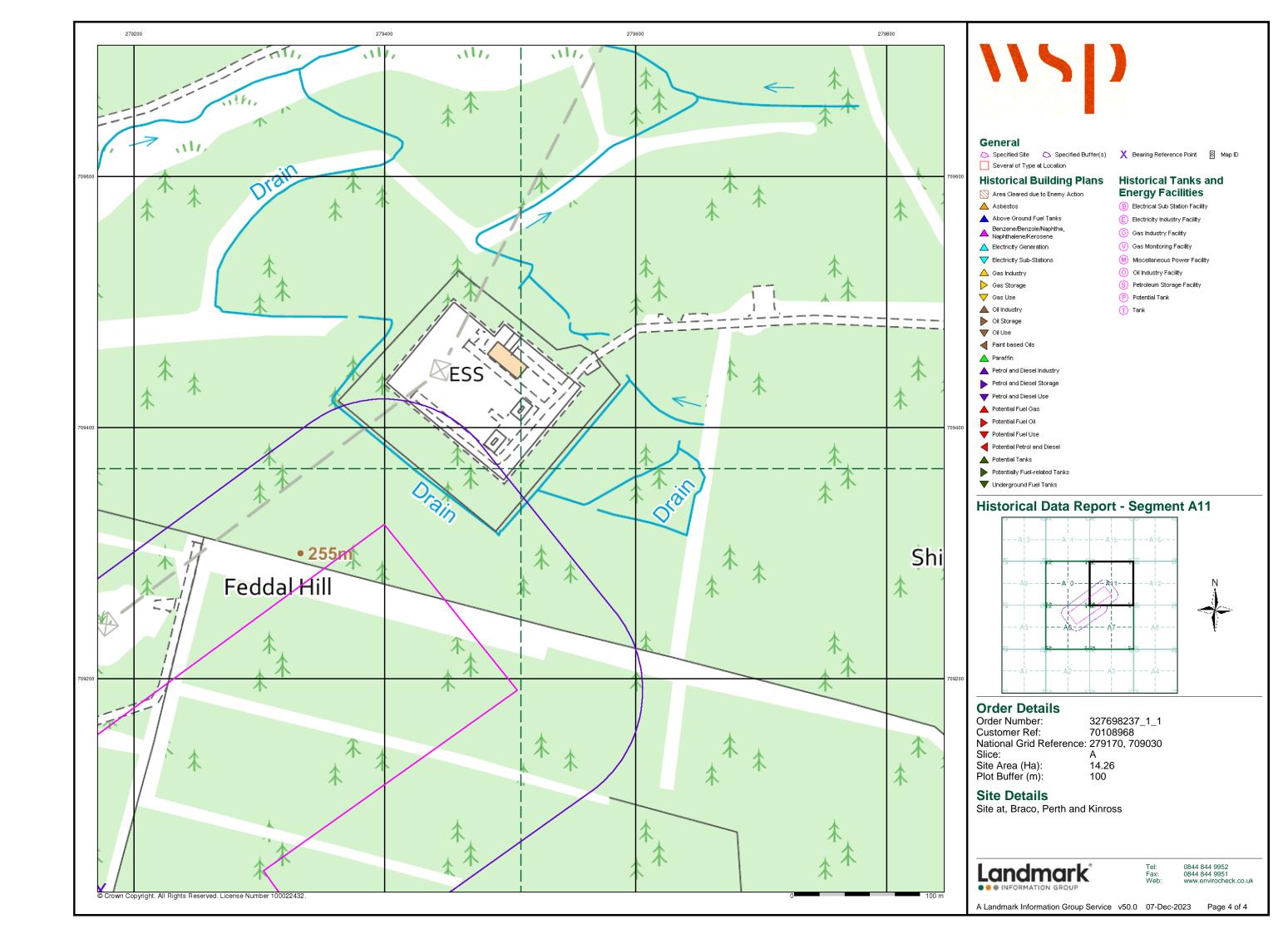
In addition to HLUD, additional analysis uncovered some of the most dangerous sources of contamination (past and present tanks, petrol storage, oil, gas, electricity, miscellaneous facilities). This data set covers over 390,000 Historical Tanks and Energy facilities in Great Britain and was captured from post war 1:2500 and 1:1250 Ordnance Survey historical mapping covering a period from 1943 to 1996.

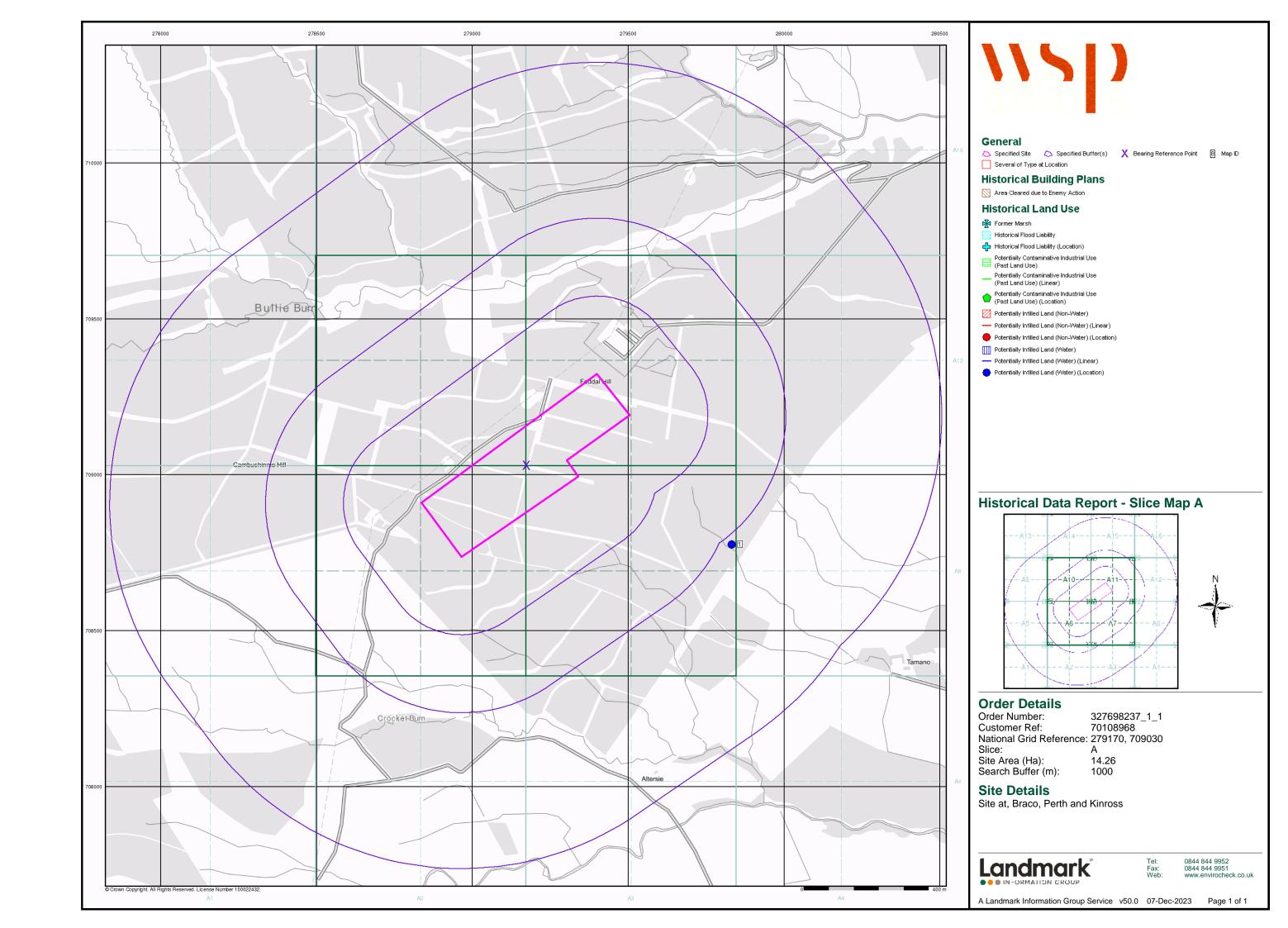
Order Number: 327698237_1_1 Date: 07-Dec-2023 rpr_ec_datasheet v53.0 A Landmark Information Group Service Page 3 of 3











Historical Mapping Legends

Gravel Pit Other Orchard Mixed Wood Deciduous Brushwood Furze Rough Pasture Arrow denotes Trigonometrical flow of water Station Site of Antiquities Bench Mark Pump, Guide Post, Well, Spring, Signal Post **Boundary Post** · 285 Surface Level Sketched Instrumental Contour Contour Fenced Main Roads Minor Roads Un-Fenced Raised Road Sunken Road Railway over Road over Railway Ri∨er Railway over Level Crossing Road over Road over Road over County Boundary (Geographical) County & Civil Parish Boundary Administrative County & Civil Parish Boundary County Borough Boundary (England) Co. Boro. Bdy. County Burgh Boundary (Scotland) Rural District Boundary R.D. Bdy.

····· Civil Parish Boundary

Ordnance Survey County Series 1:10,560

Ordnance Survey Plan 1:10,000

Errange Comments	Chalk Pit, Clay Pi	0 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0 0	Gravel Pit
	Sand Pit	()	、 Disused Pit ✓ or Quarry
(.0.0.0.0.0.0.0.0.0.0.0.0.0.0.0.0.0.0.0	Refuse or Slag Heap	((()	Lake, Loch or Pond
	Dunes	000	Boulders
* * 4	Coniferous Trees	$\Diamond \Diamond \Diamond$	Non-Coniferous Trees
ф ф	Orchard no_	Scrub	\Υ _n ν Coppice
ជា ជា ជា	Bracken	Heath '	、 , , , , Rough Grassland
<u> </u>	MarshV///	Reeds	스 <u>노</u> 스 Saltings
	Dire Building	ction of Flow of V	Shingle
***	Glasshouse	<i>"</i>	Sand
	Sloping Masonry	Pylon — — — — - Pole — — • — -	Electricity Transmission Line
***	Embankr	ment	_ Standard Gauge Multiple Track Standard Gauge
Road ' ' Under	'∏''' Road Lev Over Cros		Single Track Siding, Tramway
			or Mineral Line + Narrow Gauge
			T Namow Gauge
		County, County B	orough
	or County of Cit Municipal Borot Burgh or Distric	- ugh, Urban or Rui	ral District,
	Borough, Burgh	or County Cons	
	Civil Parish	when coincidence o	
BP, BS	Boundary Post or Stone		Police Station
Ch	Church		Post Office
CH F F Ot-	Club House		Public Convenience
F E Sta	Fire Engine Station		Public House
FB Fn	Foot Bridge Fountain		Signal Box
FII		Spr S	Spring

TCB

TCP

Guide Post

Mile Post

Telephone Call Box

Telephone Call Post

1:10,000 Raster Mapping

	Gravel Pit		Refuse tip or slag heap
	Rock	3	Rock (scattered)
	Boulders		Boulders (scattered)
	Shingle	Mud	Mud
Sand	Sand		Sand Pit
********	Slopes		Top of cliff
	General detail		Underground detail
	- O∨erhead detail		Narrow gauge railway
	Multi-track railway		Single track railway
_•-•	County boundary (England only)	• • • • • •	Ci∨il, parish or community boundary
	District, Unitary, Metropolitan, London Borough boundary		Constituency boundary
۵ ^۵	Area of wooded vegetation	\$\partial \chi \chi \chi \chi \chi \chi \chi \chi	Non-coniferous trees
\Diamond	Non-coniferous trees (scattered)	**	Coniferous trees
		** **	
۵ *	trees (scattered) Coniferous	**	trees Positioned
* *	trees (scattered) Coniferous trees (scattered)		trees Positioned tree Coppice
\$ \$\phi \ \phi \phi	trees (scattered) Coniferous trees (scattered) Orchard Rough	£ € € € € € € € € € € € € € € € € € € €	trees Positioned tree Coppice or Osiers
\$ \$ \$ \$ \$ \$ \$ \$ \$ \$ \$ \$ \$ \$ \$ \$ \$ \$ \$	trees (scattered) Coniferous trees (scattered) Orchard Rough Grassland	£ £ € € € € € € € € € € € € € € € € € €	trees Positioned tree Coppice or Osiers Heath Marsh, Salt
\$ \$ \$ \$ \$ \$ \$ \$ \$ \$ \$ \$ \$ \$ \$ \$ \$ \$ \$	trees (scattered) Coniferous trees (scattered) Orchard Rough Grassland Scrub	£ £ € € € € € € € € € € € € € € € € € €	trees Positioned tree Coppice or Osiers Heath Marsh, Salt Marsh or Reeds
\$ \$ \$ \$ \$ \$ \$ \$ \$ \$ \$ \$ \$ \$ \$ \$ \$ \$ \$	trees (scattered) Coniferous trees (scattered) Orchard Rough Grassland Scrub Water feature Mean high	ΩΩ *** ΩΩ ** ΩΩ *** ΩΩ ** ΩΩ *** ΩΩ ** ΩΩ *** ΩΩ *** ΩΩ *** ΩΩ *** ΩΩ *** ΩΩ *** ΩΩ ** Ω	trees Positioned tree Coppice or Osiers Heath Marsh, Salt Marsh or Reeds Flow arrows Mean low
\$ \$ \$ \$ \$ \$ \$ \$ \$ \$ \$ \$ \$ \$ \$ \$ \$ \$ \$	trees (scattered) Coniferous trees (scattered) Orchard Rough Grassland Scrub Water feature Mean high water (springs) Telephone line (where shown) Bench mark (where shown)	ΩΩ *** ΩΩ ** ΩΩ *** ΩΩ ** ΩΩ *** ΩΩ ** ΩΩ *** ΩΩ *** ΩΩ *** ΩΩ *** ΩΩ *** ΩΩ *** ΩΩ ** Ω	trees Positioned tree Coppice or Osiers Heath Marsh, Salt Marsh or Reeds Flow arrows Mean low water (springs) Electricity transmission line
↑ ↑ ↑ ↑ ↑ ↑ ↑ ↑ ↑ ↑ ↑ ↑ ↑ ↑ ↑ ↑ ↑ ↑ ↑	trees (scattered) Coniferous trees (scattered) Orchard Rough Grassland Scrub Water feature Mean high water (springs) Telephone line (where shown) Bench mark	ΔΩ ** ** ** ** ** ** ** ** **	trees Positioned tree Coppice or Osiers Heath Marsh, Salt Marsh or Reeds Flow arrows Mean low water (springs) Electricity transmission line (with poles) Triangulation
↑ ↑ ↑ ↑ ↑ ↑ ↑ ↑ ↑ ↑ ↑ ↑ ↑ ↑ ↑ ↑ ↑ ↑ ↑	trees (scattered) Coniferous trees (scattered) Orchard Rough Grassland Scrub Water feature Mean high water (springs) Telephone line (where shown) Bench mark (where shown) Point feature (e.g. Guide Post	∴ ∴ ∴ ∴ ∴ ∴ ∴ ∴ ∴ ∴ ∴ ∴ ∴ ∴ ∴ ∴ ∴ ∴ ∴	trees Positioned tree Coppice or Osiers Heath Marsh, Salt Marsh or Reeds Flow arrows Mean low water (springs) Electricity transmission line (with poles) Triangulation station Pylon, flare stack

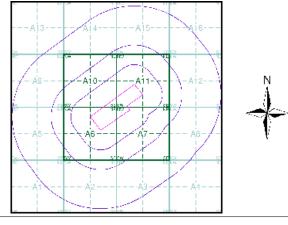
General Building



Historical Mapping & Photography included:

Mapping Type	Scale	Date	Pg
Perthshire	1:10,560	1866	2
Perthshire	1:10,560	1901	3
Ordnance Survey Plan	1:10,000	1958 - 1959	4
Ordnance Survey Plan	1:10,000	1978 - 1979	5
Ordnance Survey Plan	1:10,000	1982	6
10K Raster Mapping	1:10,000	2001	7
10K Raster Mapping	1:10,000	2006	8
VectorMap Local	1:10,000	2023	9

Historical Map - Slice A



Order Details

Order Number: 327698237_1_1 Customer Ref: 70108968 National Grid Reference: 279170, 709030 Slice:

Important

Building

Site Area (Ha): 14.26 Search Buffer (m): 1000

Site Details

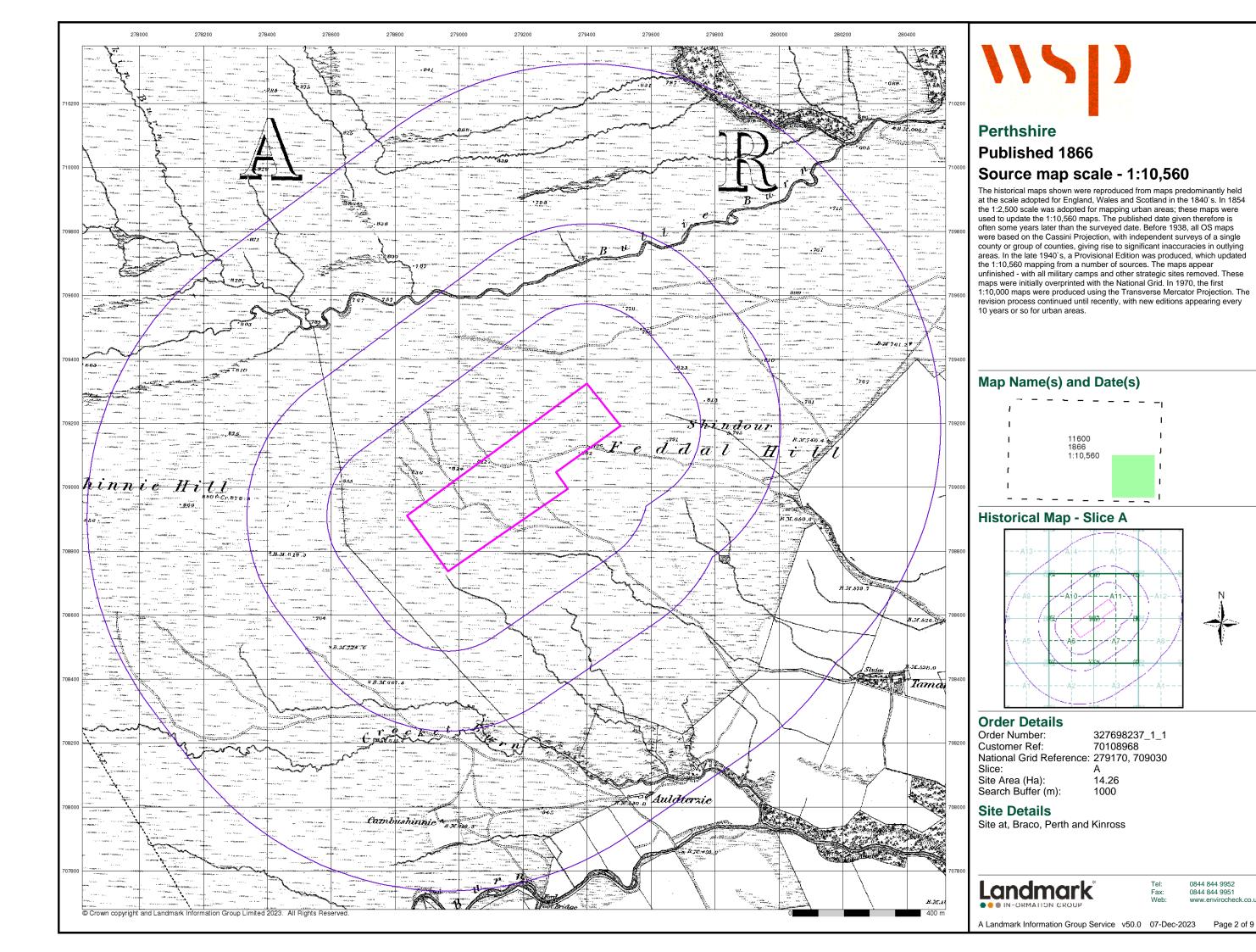
Site at, Braco, Perth and Kinross

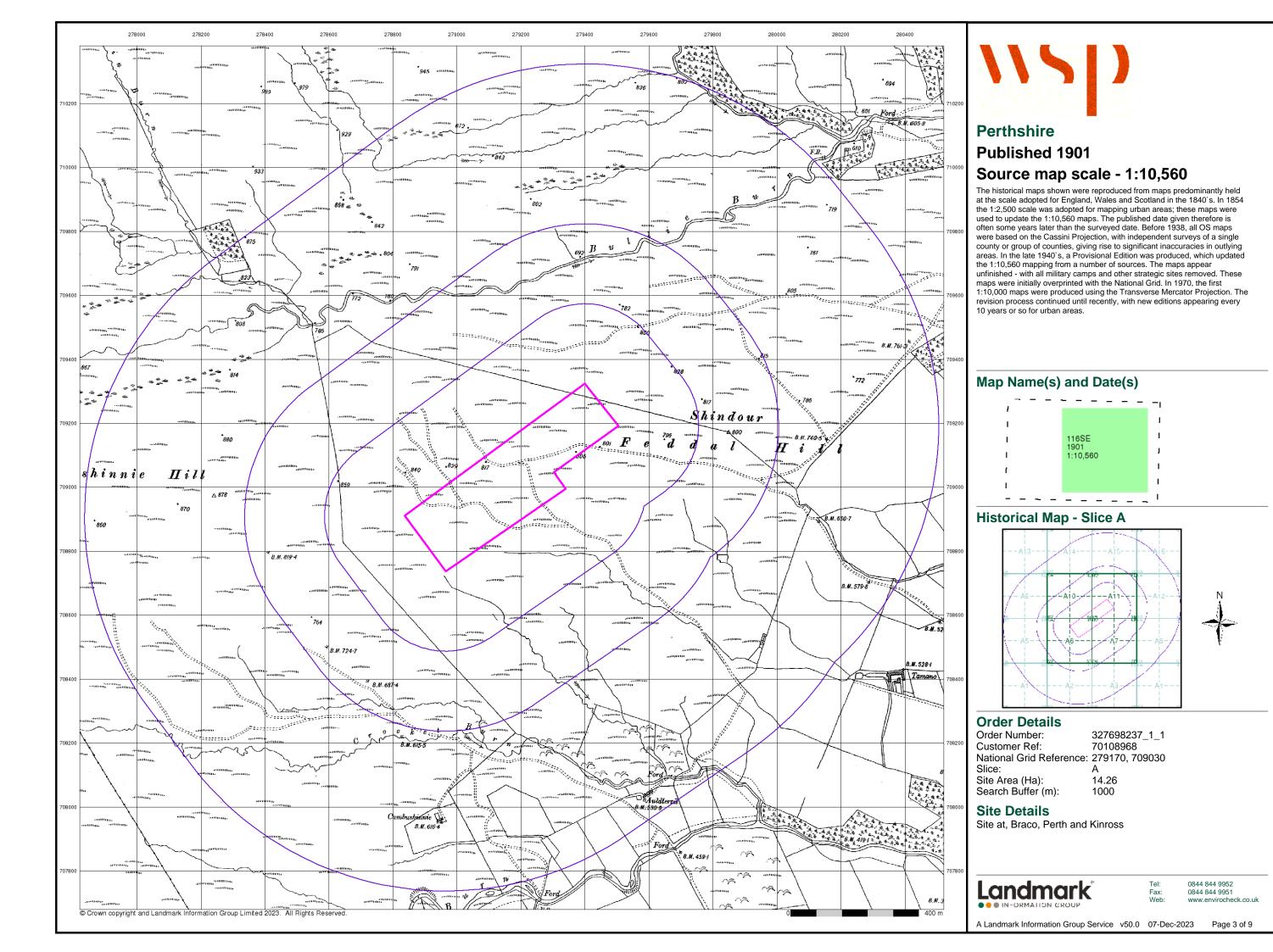


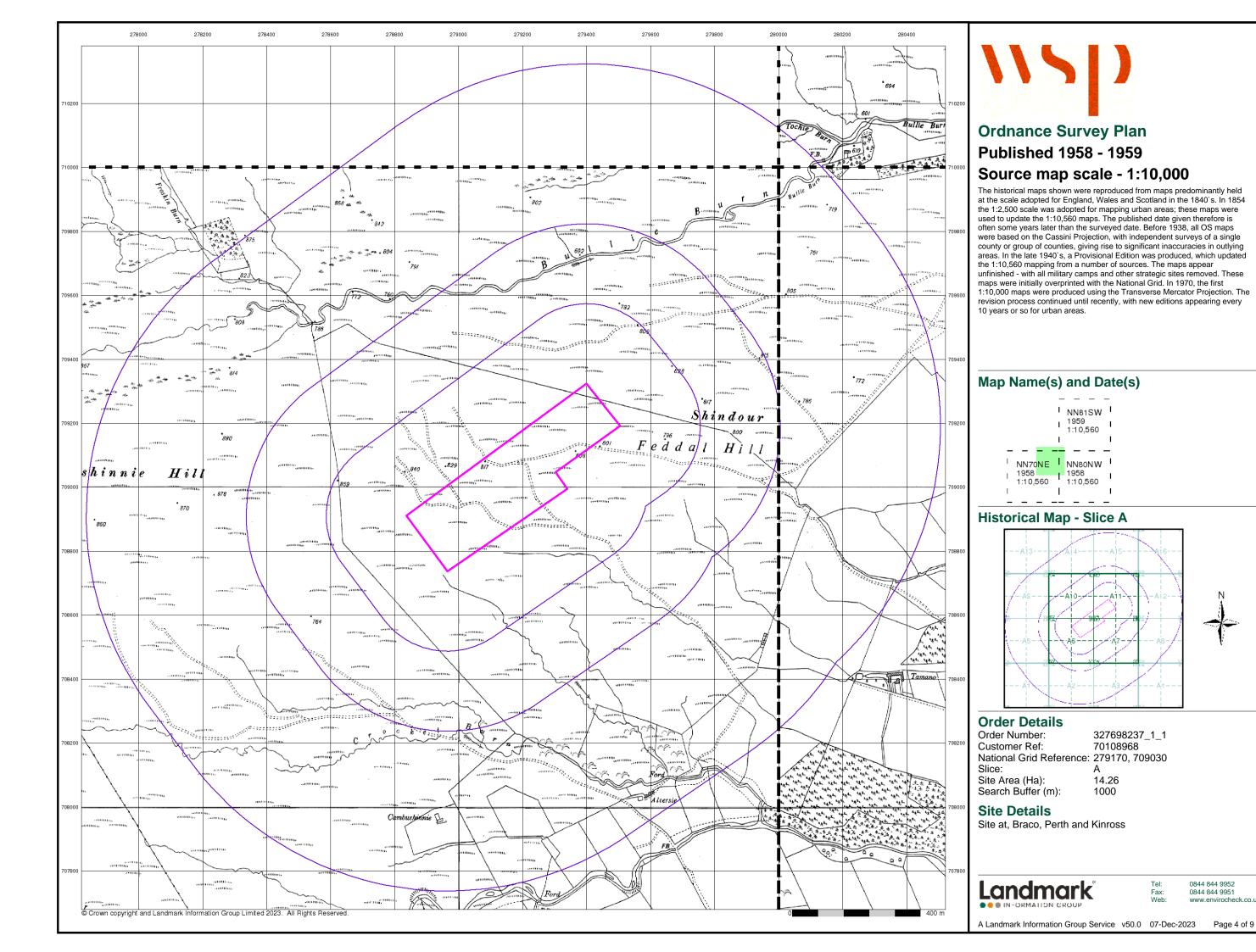
0844 844 9952 www.envirocheck.co.uk

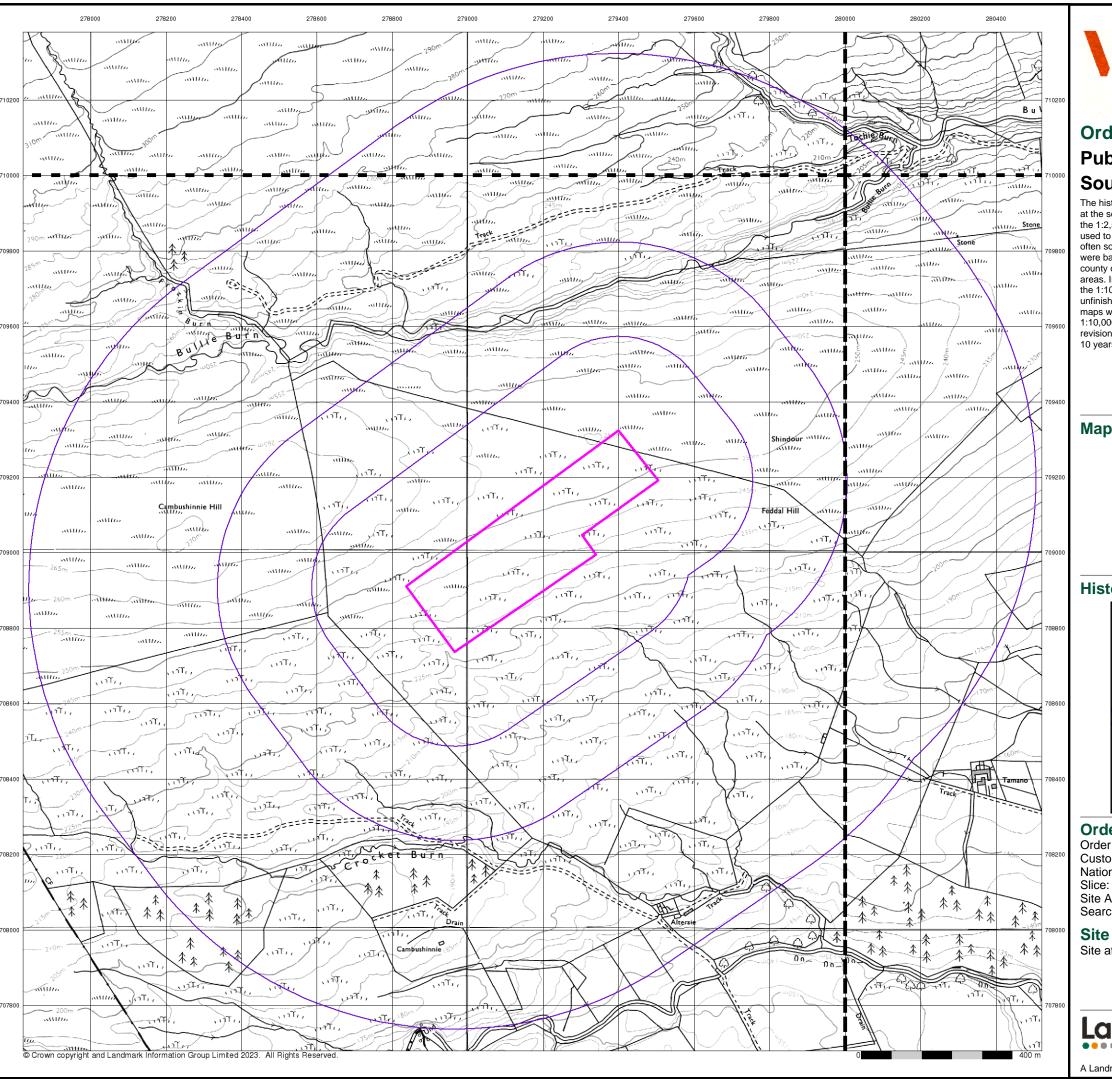
Page 1 of 9

A Landmark Information Group Service v50.0 07-Dec-2023











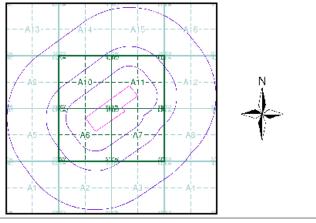
Ordnance Survey Plan Published 1978 - 1979 Source map scale - 1:10,000

The historical maps shown were reproduced from maps predominantly held at the scale adopted for England, Wales and Scotland in the 1840's. In 1854 the 1:2,500 scale was adopted for mapping urban areas; these maps were used to update the 1:10,560 maps. The published date given therefore is often some years later than the surveyed date. Before 1938, all OS maps were based on the Cassini Projection, with independent surveys of a single county or group of counties, giving rise to significant inaccuracies in outlying areas. In the late 1940's, a Provisional Edition was produced, which updated the 1:10,560 mapping from a number of sources. The maps appear unfinished - with all military camps and other strategic sites removed. These maps were initially overprinted with the National Grid. In 1970, the first 1:10,000 maps were produced using the Transverse Mercator Projection. The revision process continued until recently, with new editions appearing every 10 years or so for urban areas.

Map Name(s) and Date(s)

_	_	_		_	_	_
I	NN71	SE	I	NN8	1SW	· I
1	1978 1:10,0	າດດ	1	1978 1:10.		I
I	1.10,	500	1	10	000	1
_	_			_	_	_
1	NN70	NE	1	NN8	WNC	ı
1	1978 1:10,0	200	1	1979 1:10.		1
		000			000	
			ı			ı

Historical Map - Slice A



Order Details

Order Number: 327698237_1_1
Customer Ref: 70108968
National Grid Reference: 279170, 709030

41.

Site Area (Ha): 14.26 Search Buffer (m): 1000

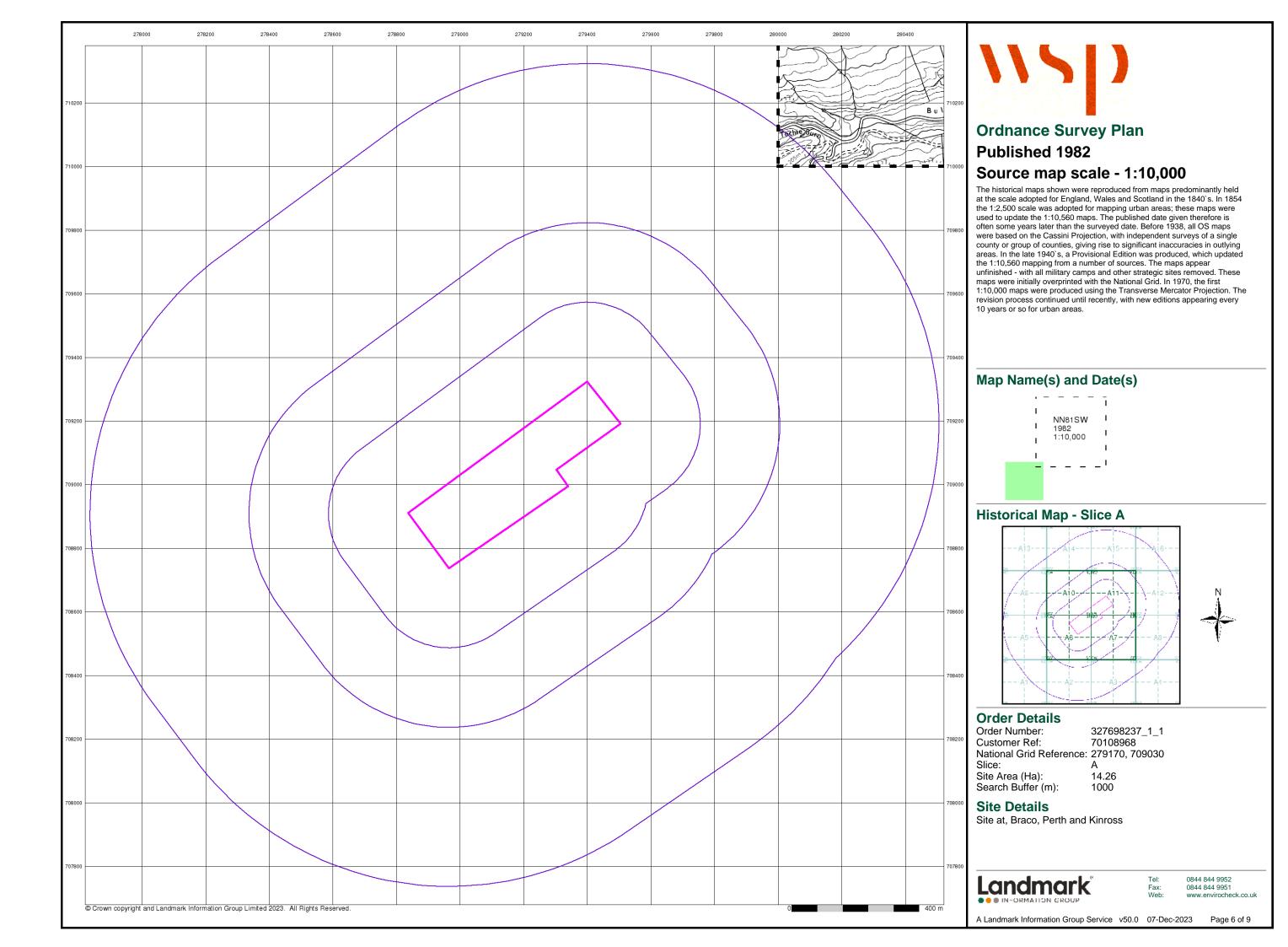
Site Details

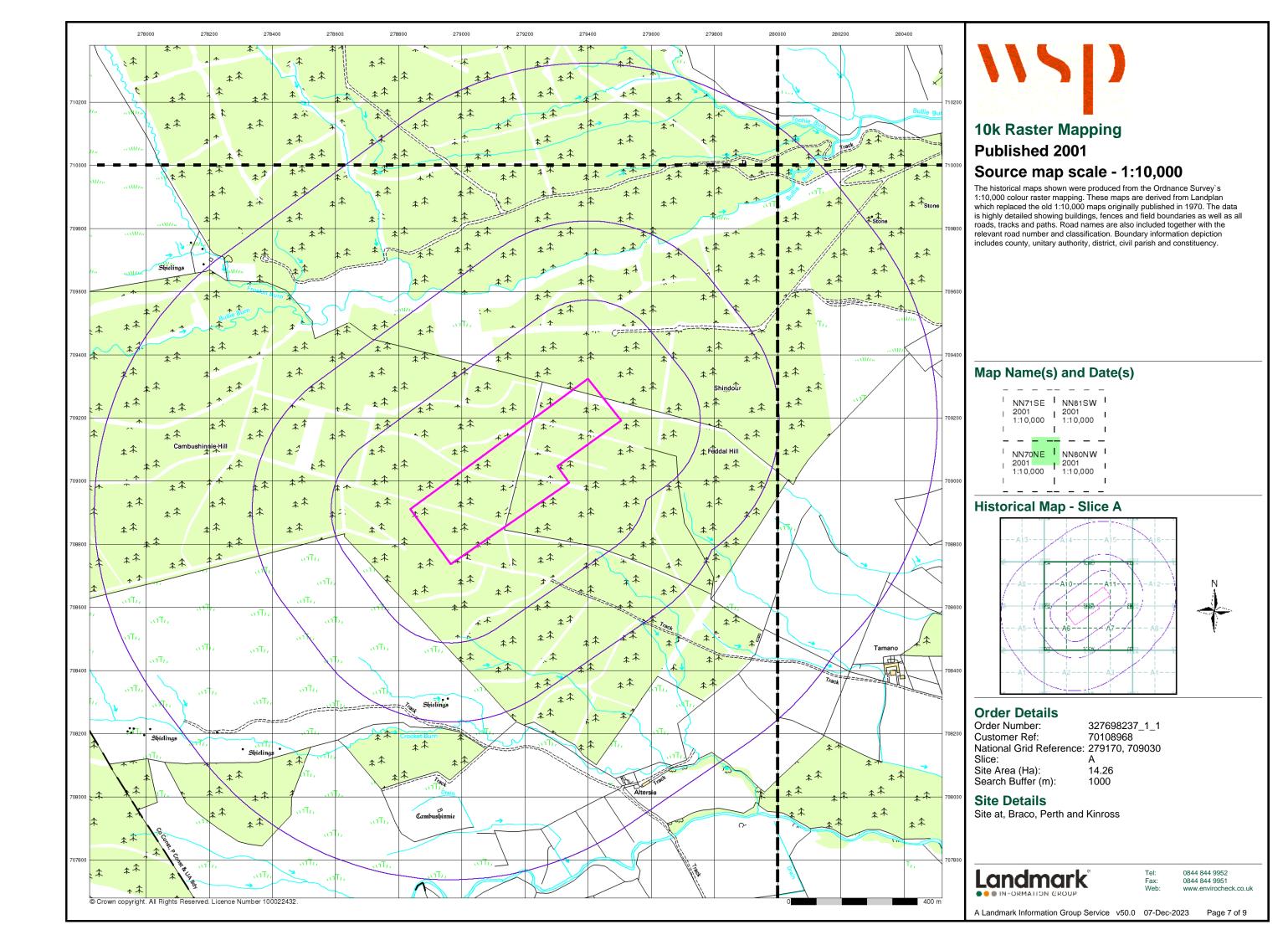
Site at, Braco, Perth and Kinross

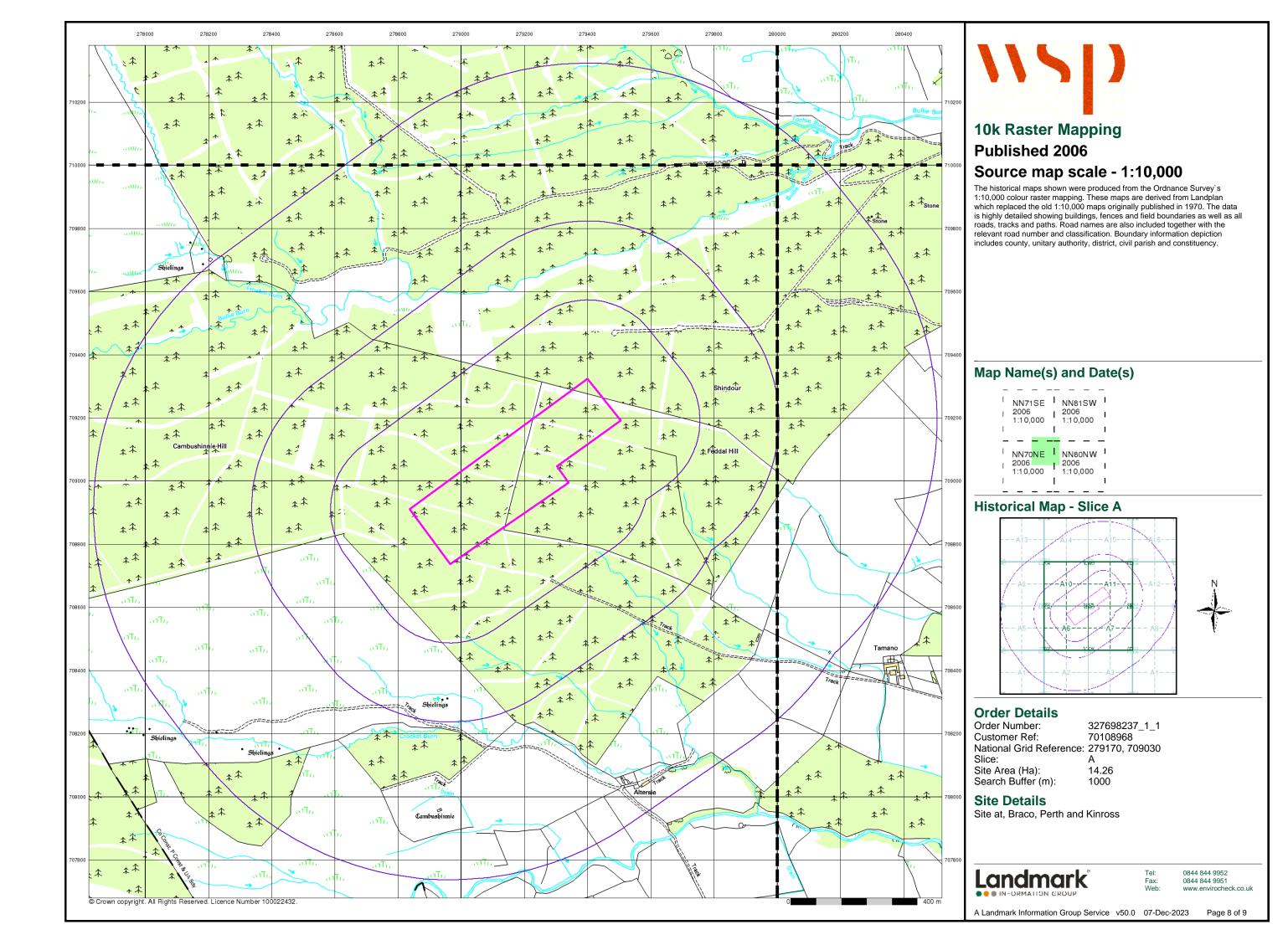
Landmark*

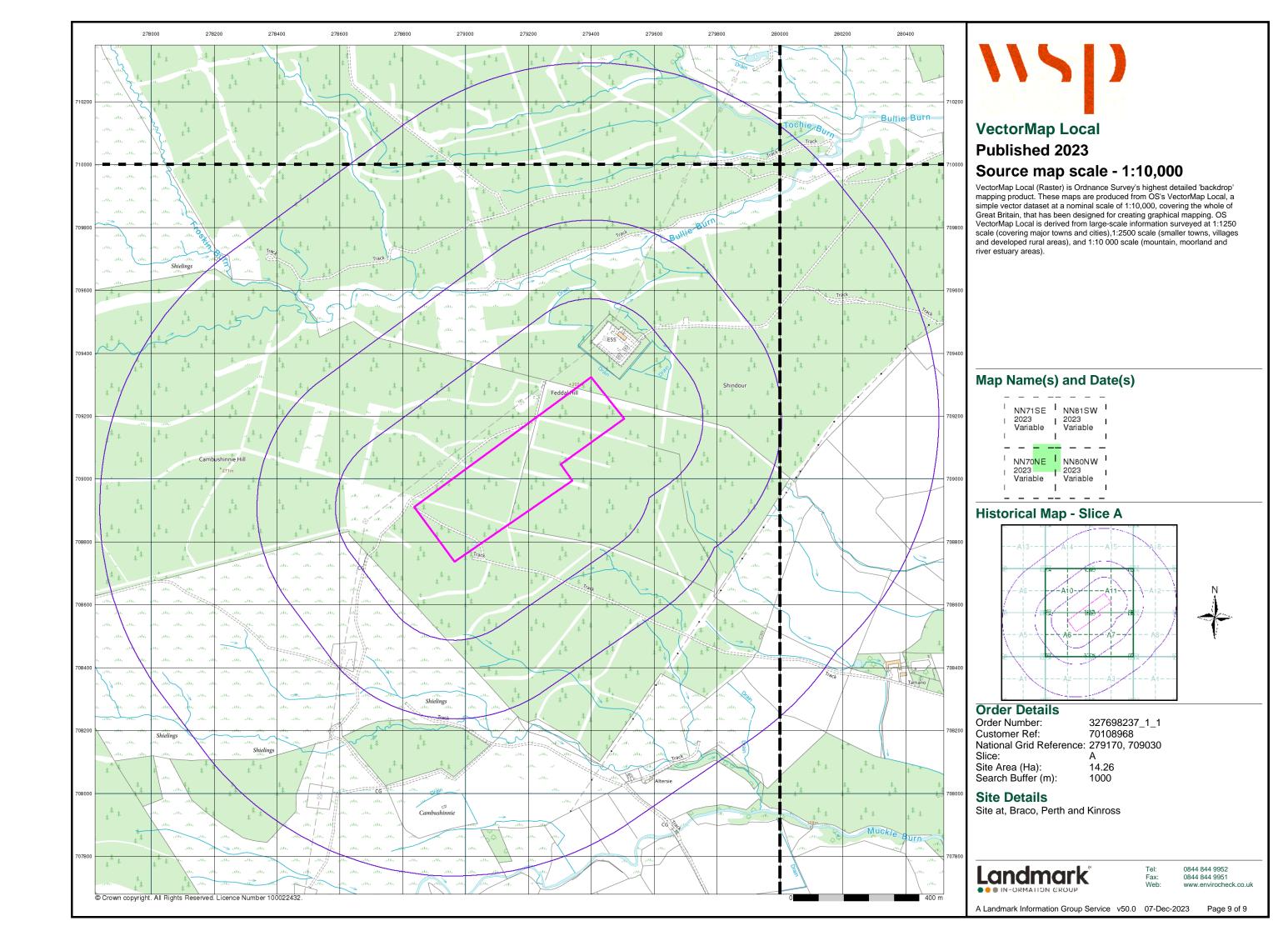
Tel: 0844 844 9952 Fax: 0844 844 9951 Web: www.envirocheck

A Landmark Information Group Service v50.0 07-Dec-2023 Page 5 of 9

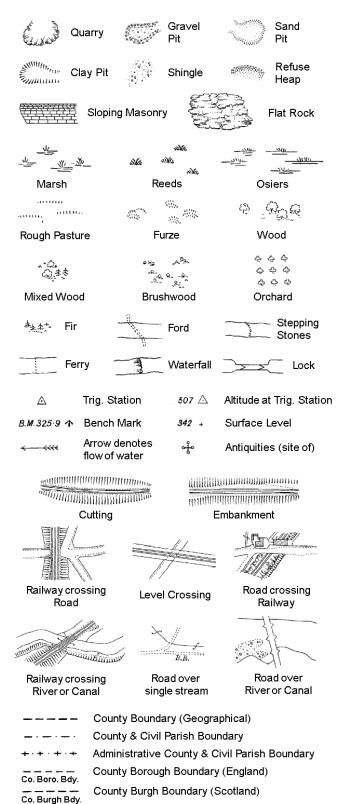








Ordnance Survey County Series and Ordnance Survey Plan 1:2,500



B.R.

E.P

F.B.

M.S

Bridle Road

Foot Bridge

Mile Stone

M.P.M.R. Mooring Post or Ring

Electricity Pylor

Guide Post or Board

Police Call Box

Telephone Call Box

Signal Post

Pump

Sluice

Spring

Trough

Well

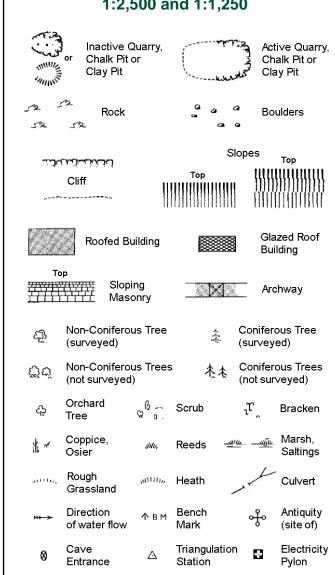
S.P

T.C.B

Sl.

 T_T

Ordnance Survey Plan, Additional SIMs and Large-Scale National Grid Data 1:2,500 and **Supply of Unpublished Survey Information** 1:2,500 and 1:1,250



County Boundary (Geographical) County & Civil Parish Boundary Civil Parish Boundary Admin. County or County Bor. Boundary L B Bdy London Borough Boundary

Electricity Transmission Line

вн	Beer House	Р	Pillar, Pole or Post
BP, BS	Boundary Post or Stone	PO	Post Office
Cn, C	Capstan, Crane	PC	Public Convenience
Chy	Chimney	PH	Public House
D Fn	Drinking Fountain	Pp	Pump
EIP	Electricity Pillar or Post	SB, S Br	Signal Box or Bridge
FAP	Fire Alarm Pillar	SP, SL	Signal Post or Light
FB	Foot Bridge	Spr	Spring
GP	Guide Post	Tk	Tank or Track
Н	Hydrant or Hydraulic	TCB	Telephone Call Box
LC	Level Crossing	TCP	Telephone Call Post
MH	Manhole	Tr	Trough
MP	Mile Post or Mooring Post	WrPt,WrT	Water Point, Water Tap
MS	Mile Stone	W	Well
NTL	Normal Tidal Limit	Wd Pp	Wind Pump

mereing changes

Symbol marking point where boundary

Fn/DFn

GVC

MP, MS

Fountain / Drinking Ftn.

Gas Valve Compound

Mile Post or Mile Stone

Gas Governer

Guide Post

Manhole

Tk

Tr

Wd Pp

Wks

Tank or Track

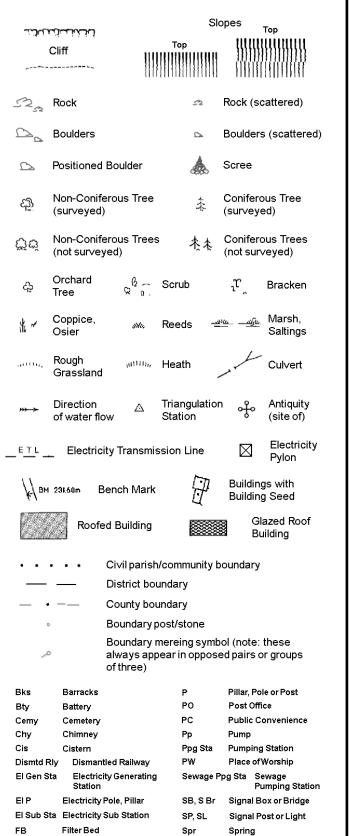
Trough

Wind Pump

Wr Pt. Wr T Water Point, Water Tap

Works (building or area)

1:1,250

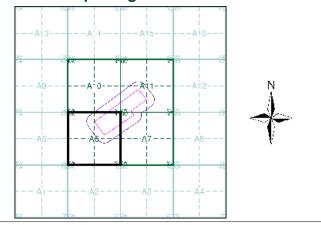




Historical Mapping & Photography included:

Mapping Type	Scale	Date	Pa
Historical Aerial Photography	1:2,500	2005	2

Historical Map - Segment A6



Order Details

Order Number: 327698237_1_1 70108968 Customer Ref: National Grid Reference: 279170, 709030 Slice:

Site Area (Ha): 14.26 Search Buffer (m): 100

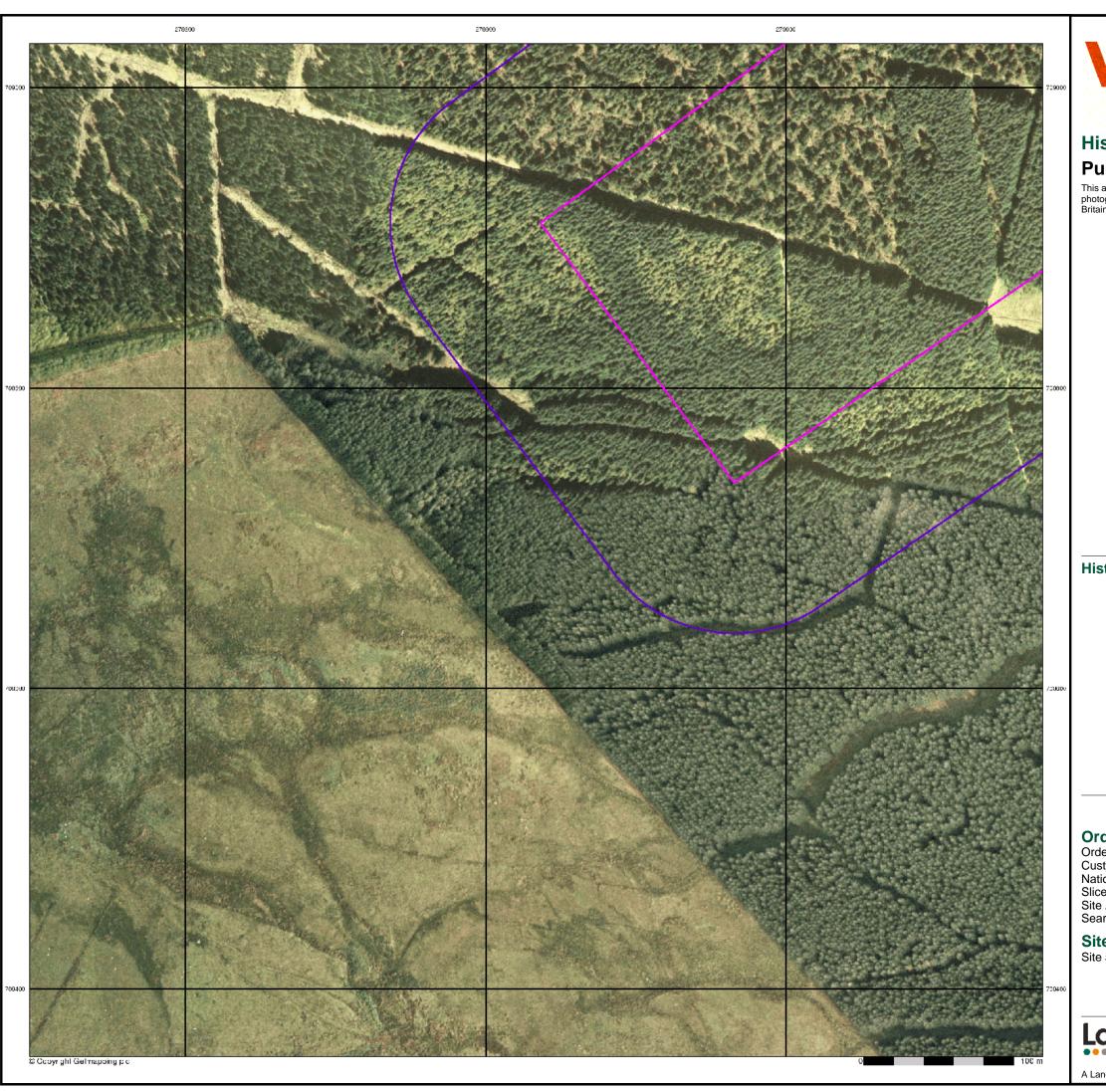
Site Details

Site at, Braco, Perth and Kinross



0844 844 9952 0844 844 9951

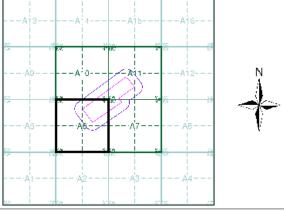
A Landmark Information Group Service v50.0 07-Dec-2023 Page 1 of 2

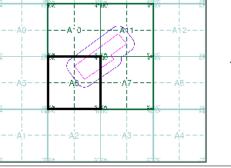




This aerial photography was produced by Getmapping, these vertical aerial photographs provide a seamless, full colour survey of the whole of Great Britain

Historical Aerial Photography - Segment A6





Order Details
Order Number: Order Number: 327698237_1_1
Customer Ref: 70108968
National Grid Reference: 279170, 709030

Slice: Site Area (Ha): Search Buffer (m): 14.26 100

Site Details

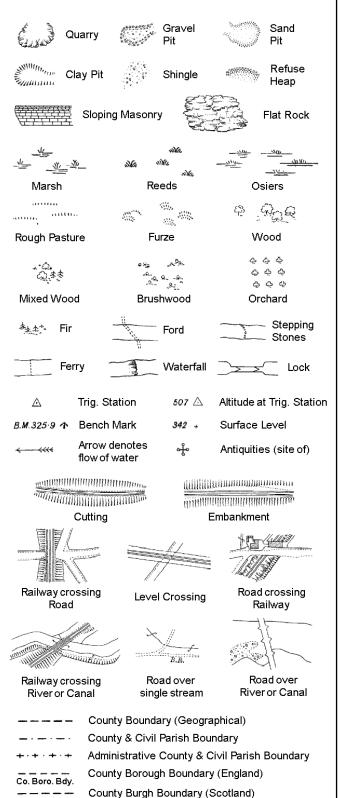
Site at, Braco, Perth and Kinross

Landmark*

0844 844 9952 0844 844 9951 www.envirocheck.co.uk

A Landmark Information Group Service v50.0 07-Dec-2023 Page 2 of 2

Ordnance Survey County Series and Ordnance Survey Plan 1:2,500



Police Call Box

Telephone Call Box

Signal Post

Pump

Sluice

Spring

Trough

Well

S.P

T.C.B

Sl.

Tr

Co. Burgh Bdy.

Bridle Road

Foot Bridge

Mile Stone

M.P.M.R. Mooring Post or Ring

Electricity Pylor

Guide Post or Board

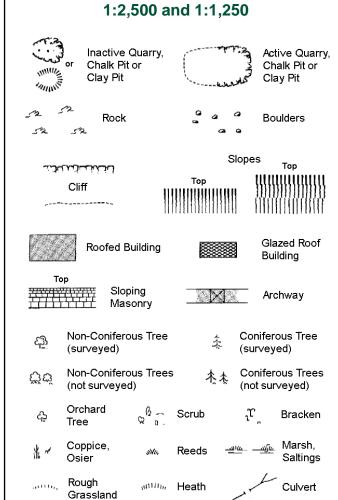
B.R.

E.P

F.B.

M.S

Ordnance Survey Plan, Additional SIMs and Large-Scale National Grid Data 1:2,500 and **Supply of Unpublished Survey Information**

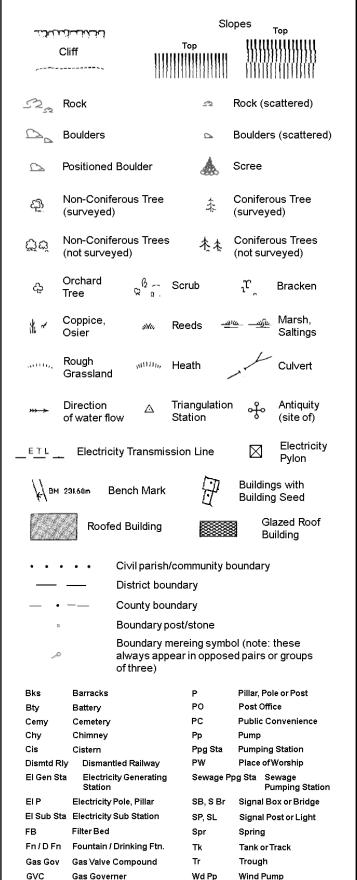


Direction Bench Antiquity of water flow (site of) Electricity Cave Triangulation Entrance

ETL Elec	tricity Transmission Line
	County Boundary (Geographical)
. — . — .	County & Civil Parish Boundary
	Civil Parish Boundary
· · ·	Admin. County or County Bor. Boundary
-e- L B Bdy -e-	London Borough Boundary
	Symbol marking point where boundary mereing changes

вн	Beer House	Р	Pillar, Pole or Post
BP, BS	Boundary Post or Stone	PO	Post Office
Cn, C	Capstan, Crane	PC	Public Convenience
Chy	Chimney	PH	Public House
D Fn	Drinking Fountain	Pp	Pump
EIP	Electricity Pillar or Post	SB, S Br	Signal Box or Bridge
FAP	Fire Alarm Pillar	SP, SL	Signal Post or Light
FB	Foot Bridge	Spr	Spring
GP	Guide Post	Tk	Tank or Track
Н	Hydrant or Hydraulic	TCB	Telephone Call Box
LC	Level Crossing	TCP	Telephone Call Post
MH	Manhole	Tr	Trough
MP	Mile Post or Mooring Post	WrPt,WrT	Water Point, Water Tap
MS	Mile Stone	W	Well
NTL	Normal Tidal Limit	Wd Pp	Wind Pump

1:1,250

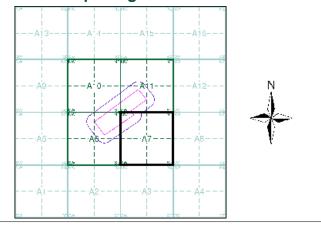




Historical Mapping & Photography included:

Mapping Type	Scale	Date	Pg
Historical Aerial Photography	1:2,500	2005	2

Historical Map - Segment A7



Order Details

Order Number: 327698237_1_1 70108968 Customer Ref: National Grid Reference: 279170, 709030 Slice:

Wr Pt. Wr T Water Point, Water Tap

Works (building or area)

Wks

Guide Post

Mile Post or Mile Stone

Manhole

MP, MS

Site Area (Ha): 14.26 Search Buffer (m): 100

Site Details

Site at, Braco, Perth and Kinross



0844 844 9952 0844 844 9951

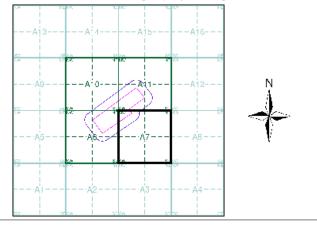
A Landmark Information Group Service v50.0 07-Dec-2023 Page 1 of 2





This aerial photography was produced by Getmapping, these vertical aerial photographs provide a seamless, full colour survey of the whole of Great Britain

Historical Aerial Photography - Segment A7



Order Details
Order Number: Order Number: 327698237_1_1
Customer Ref: 70108968
National Grid Reference: 279170, 709030

Slice: Site Area (Ha): Search Buffer (m): 14.26 100

Site Details

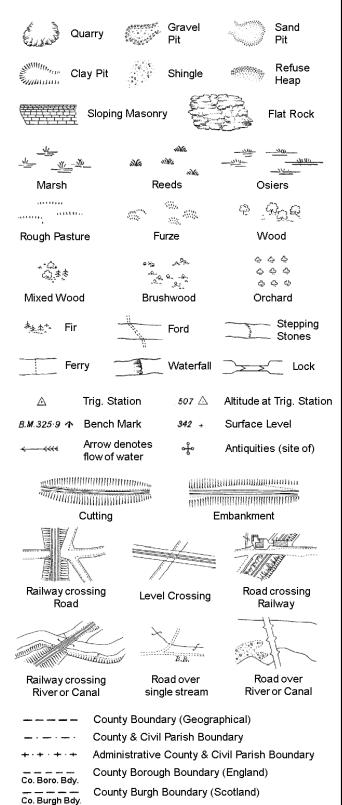
Site at, Braco, Perth and Kinross

Landmark*

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Ordnance Survey County Series and Ordnance Survey Plan 1:2,500



B.R.

E.P

F.B.

M.S

Bridle Road

Foot Bridge

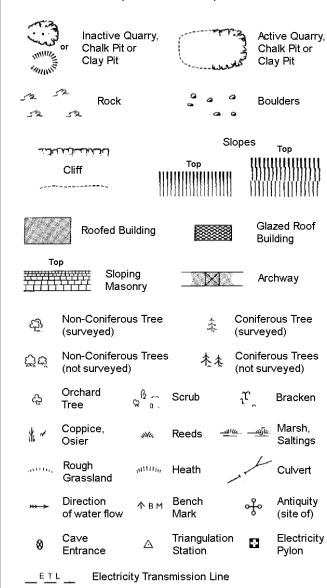
Mile Stone

M.P.M.R. Mooring Post or Ring

Electricity Pylor

Guide Post or Board

Ordnance Survey Plan, Additional SIMs and Large-Scale National Grid Data 1:2,500 and **Supply of Unpublished Survey Information** 1:2,500 and 1:1,250



14		mereing chai	nges		
	вн	Beer House	Р	Pillar, Pole or Post	
	BP, BS	Boundary Post or Stone	PO	Post Office	
	Cn, C	Capstan, Crane	PC	Public Convenience	
	Chy	Chimney	PH	Public House	
	D Fn	Drinking Fountain	Pp	Pump	
	EIP	Electricity Pillar or Post	SB, S Br	Signal Box or Bridge	
	FAP	Fire Alarm Pillar	SP, SL	Signal Post or Light	
	FB	Foot Bridge	Spr	Spring	
	GP	Guide Post	Tk	Tank or Track	
	Н	Hydrant or Hydraulic	тсв	Telephone Call Box	
	LC	Level Crossing	TCP	Telephone Call Post	
	MH	Manhole	Tr	Trough	
	MP	Mile Post or Mooring Post	WrPt,WrT	Water Point, Water Tap	
	MS	Mile Stone	W	Well	
	NTL	Normal Tidal Limit	Wd Pp	Wind Pump	

County Boundary (Geographical)

Admin. County or County Bor. Boundary

Symbol marking point where boundary

Fn/DFn

GVC

Fountain / Drinking Ftn.

Gas Valve Compound

Mile Post or Mile Stone

Gas Governer

Guide Post

Manhole

Tk

Tr

Wd Pp

Wks

Tank or Track

Trough

Wind Pump

Wr Pt. Wr T Water Point, Water Tap

Works (building or area)

County & Civil Parish Boundary

Civil Parish Boundary

London Borough Boundary

L B Bdy

Police Call Box

Telephone Call Box

Signal Post

Pump

Sluice

Spring

Trough

Well

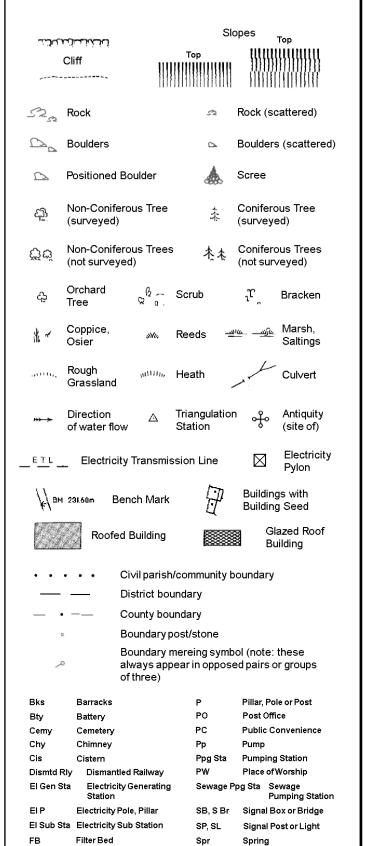
S.P

T.C.B

Sl.

 T_T

1:1,250

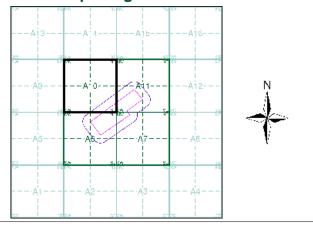




Historical Mapping & Photography included:

Mapping Type	Scale	Date	Pg
Historical Aerial Photography	1:2,500	2005	2

Historical Map - Segment A10



Order Details

Order Number: 327698237_1_1 70108968 Customer Ref: National Grid Reference: 279170, 709030 Slice:

Site Area (Ha): 14.26 Search Buffer (m): 100

Site Details

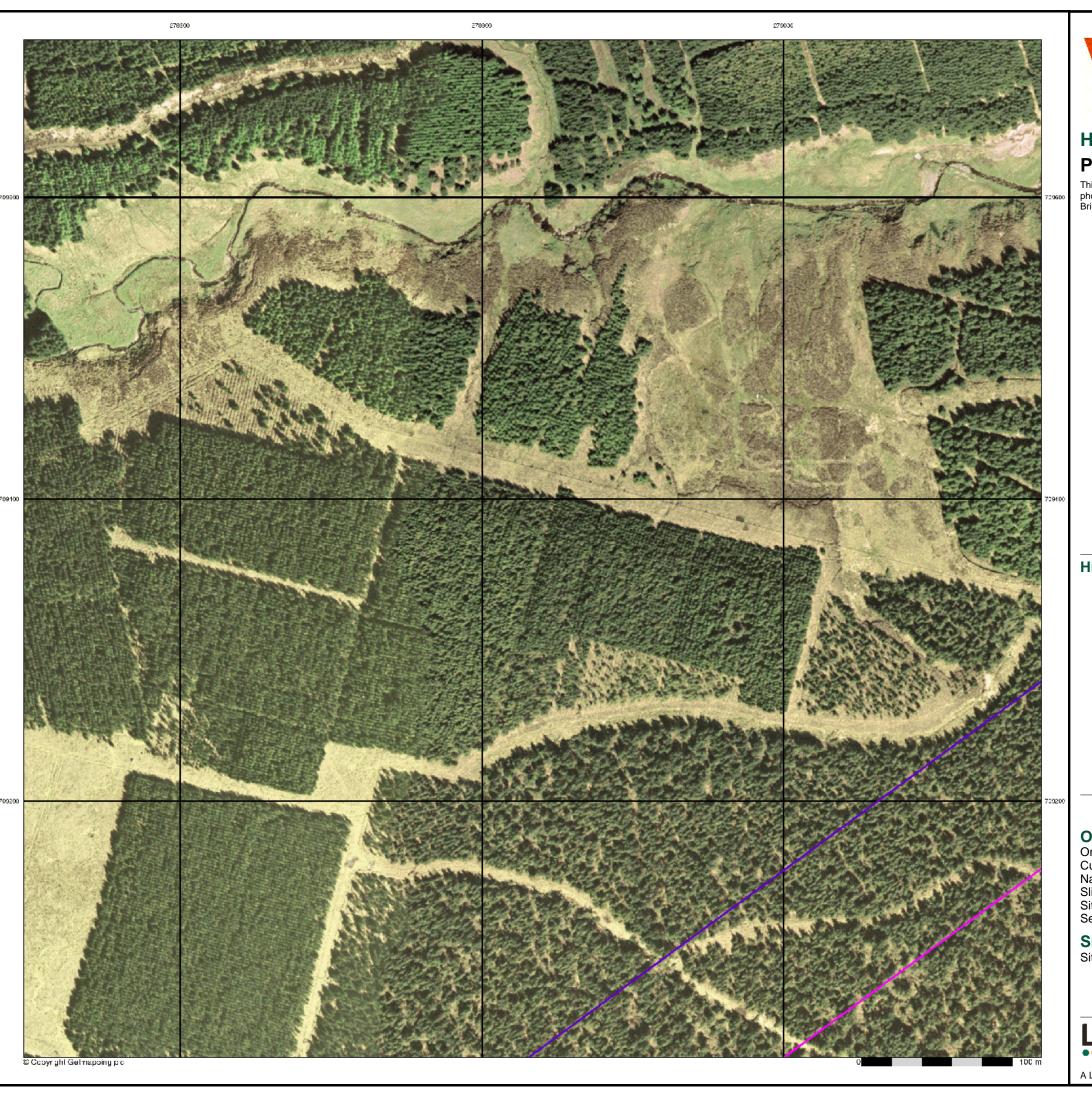
Site at, Braco, Perth and Kinross



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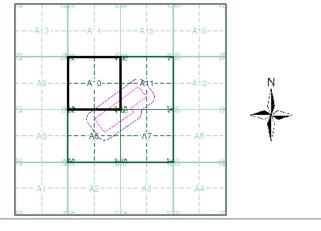
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This aerial photography was produced by Getmapping, these vertical aerial photographs provide a seamless, full colour survey of the whole of Great Britain

Historical Aerial Photography - Segment A10



Order Details
Order Number: Order Number: 327698237_1_1
Customer Ref: 70108968
National Grid Reference: 279170, 709030

Slice: Site Area (Ha): Search Buffer (m): 14.26 100

Site Details

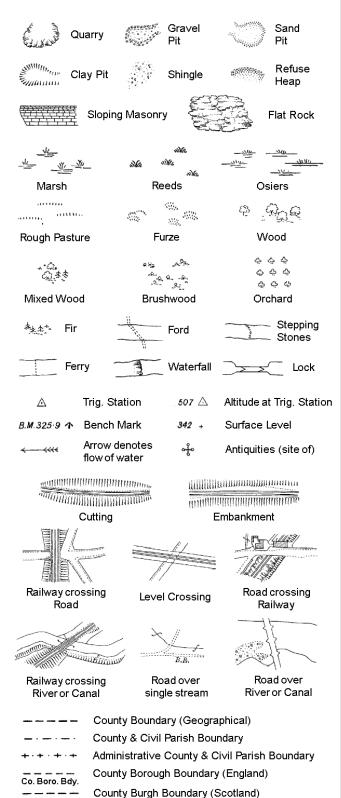
Site at, Braco, Perth and Kinross

Landmark*

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Ordnance Survey County Series and Ordnance Survey Plan 1:2,500



Police Call Box

Telephone Call Box

Signal Post

Pump

Sluice

Spring

Trough

Well

S.P

T.C.B

Sl.

Tr

Co. Burgh Bdy.

Bridle Road

Foot Bridge

Mile Stone

M.P.M.R. Mooring Post or Ring

Electricity Pylor

Guide Post or Board

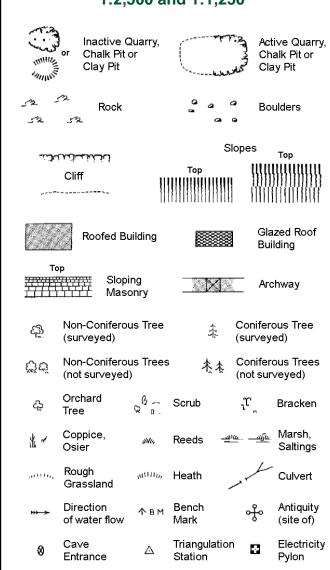
B.R.

E.P

F.B.

M.S

Ordnance Survey Plan, Additional SIMs and Large-Scale National Grid Data 1:2,500 and **Supply of Unpublished Survey Information** 1:2,500 and 1:1,250



County Boundary (Geographical) County & Civil Parish Boundary

Electricity Transmission Line

Civil Parish Boundary Admin. County or County Bor. Boundary L B Bdy London Borough Boundary Symbol marking point where boundary mereing changes

вн	Beer House	Р	Pillar, Pole or Post
BP, BS	Boundary Post or Stone	PO	Post Office
Cn, C	Capstan, Crane	PC	Public Convenience
Chy	Chimney	PH	Public House
D Fn	Drinking Fountain	Pp	Pump
EIP	Electricity Pillar or Post	SB, S Br	Signal Box or Bridge
FAP	Fire Alarm Pillar	SP, SL	Signal Post or Light
FB	Foot Bridge	Spr	Spring
GP	Guide Post	Tk	Tank or Track
Н	Hydrant or Hydraulic	TCB	Telephone Call Box
LC	Level Crossing	TCP	Telephone Call Post
MH	Manhole	Tr	Trough
MP	Mile Post or Mooring Post	WrPt,WrT	Water Point, Water Tap
MS	Mile Stone	W	Well
NTL	Normal Tidal Limit	Wd Pp	Wind Pump
	BP, BS Cn, C Chy D Fn EI P FAP FB GP H LC MH MP MS	BP, BS Boundary Post or Stone Cn, C Capstan, Crane Chy Chimney DFn Drinking Fountain EI P Electricity Pillar or Post FAP Fire Alarm Pillar FB Foot Bridge GP Guide Post H Hydrant or Hydraulic LC Level Crossing MH Manhole MP Mile Post or Mooring Post MS Mile Stone	BP, BS Boundary Post or Stone PO Cn, C Capstan, Crane PC Chy Chimney PH D Fn Drinking Fountain Pp EI P Electricity Pillar or Post SB, S Br FAP Fire Alarm Pillar SP, SL FB Foot Bridge Spr GP Guide Post Tk H Hydrant or Hydraulic TCB LC Level Crossing TCP MH Manhole Tr MP Mile Post or Mooring Post Wr Pt, Wr T MS Mile Stone W

GVC

MP, MS

Gas Governer

Mile Post or Mile Stone

Guide Post

Manhole

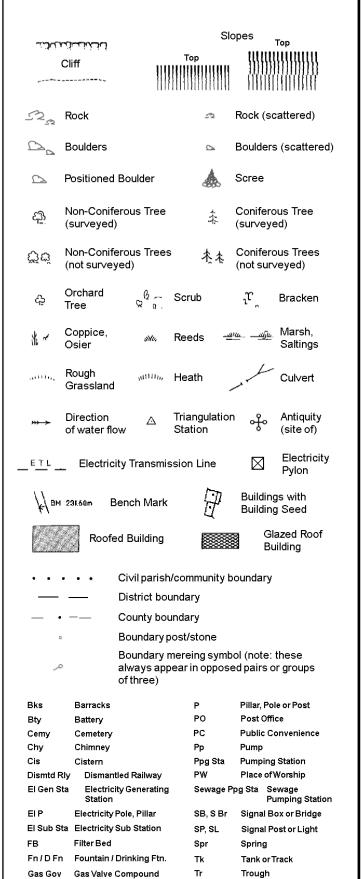
Wd Pp

Wks

Wind Pump Wr Pt. Wr T Water Point, Water Tap

Works (building or area)

1:1,250

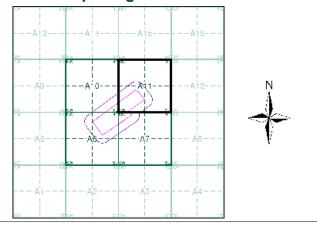




Historical Mapping & Photography included:

Mapping Type	Scale	Date	Pg
Historical Aerial Photography	1:2,500	2005	2

Historical Map - Segment A11



Order Details

Order Number: 327698237_1_1 70108968 Customer Ref: National Grid Reference: 279170, 709030 Slice:

Site Area (Ha): 14.26 Search Buffer (m): 100

Site Details

Site at, Braco, Perth and Kinross



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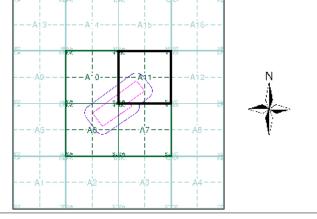
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This aerial photography was produced by Getmapping, these vertical aerial photographs provide a seamless, full colour survey of the whole of Great Britain

Historical Aerial Photography - Segment A11



Order Details
Order Number: Order Number: 327698237_1_1
Customer Ref: 70108968
National Grid Reference: 279170, 709030

Slice: Site Area (Ha): Search Buffer (m): A 14.26 100

Site Details

Site at, Braco, Perth and Kinross

Landmark*

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TECHNICAL NOTE - GEOTECHNICAL SYNOPSIS

DATE: 17 July 2024 **CONFIDENTIALITY:** Confidential

SUBJECT: Cambushinnie 400kV Substation, 70108968

CLIENT: Balfour Beatty AUTHOR: Sophie Gedge

CHECKED: Darren Warnock APPROVED: Brennig Davis

APPENDIX B PRELIMINARY EARTHWORKS DRAWINGS



TECHNICAL NOTE - GEOTECHNICAL SYNOPSIS

DATE: 17 July 2024 **CONFIDENTIALITY:** Confidential

SUBJECT: Cambushinnie 400kV Substation, 70108968

CLIENT: Balfour Beatty **AUTHOR:** Sophie Gedge

CHECKED: Darren Warnock APPROVED: Brennig Davis

APPENDIX C GEOTECHNICAL RISK REGISTER

&

GEOTECHNICAL ASSUMPTIONS REGISTER



TECHNICAL NOTE

DATE: 17 July 2024 CONFIDENTIALITY: Confidential

SUBJECT: Cambushinnie 400kV Substation, 70108968

CLIENT: Balfour Beatty AUTHOR: Sophie Gedge

CHECKED: Darren Warnock APPROVED: Brennig Davis

APPENDIX C - GEOTECHNICAL RISK REGISTER AND ASSUMPTIONS REGISTER

Table C-1 presents a Geotechnical Risk Register for the proposed works; Table C-2 presents a Geotechnical Assumptions Register for the proposed works.

The assessment of Geotechnical Risk partly fulfils the requirements of the 'Designer' as defined by the Construction (Design and Management) Regulations 2015 and considers the geotechnical risks associated with: carrying out design, during construction, maintenance, and as part of demolition work, taking into account the requirements of the Workplace (Health, Safety and Welfare) Regulations 1992. Assessed risks include consideration of risk to the project in general (i.e. time, cost etc.).

The risks discussed in **Table C-1** have been considered following a review of the available data only. These risks shall form the basis of a 'live' risk register document as defined in and maintained for the lifetime of the project, updated on every review of geotechnical risks. The risks discussed consider the risks that are impacted by or impact on the design of the proposed works discussed in this Technical Note.

The Risk Grading in **Table C-1** is given both before and after mitigation.

Table C-1 – Geotechnical Risk Register

Risk Ref.	Risk	Works Affected	Details	Likelihood (Severity)	Risk Grading	Mitigation	Likelihood (Severity) After mitigation	Risk Grading After mitigation
1	Lack of ground investigation information below the proposed site leading to inappropriate ground model and material properties	Excavations, Earthworks, Foundations	Ground investigation undertaken by SSE has widely spaced exploratory holes. Assumptions made for purposes of design may results in inaccurate results or excessive design requirements (i.e. deepening of foundation block).	Likely (Serious)	Moderate	GI proposed spread over the whole site area with allowance for geotechnical testing. The encountered ground conditions should be reviewed and characteristic values of parameters should be determined where appropriate, based on in situ and laboratory test results. The encountered ground conditions should be used to inform proposed founding depths for foundations, and to inform proposed slope angles. It is the responsibility of the Contractor to make appropriate cost and contingency allowances in their tender submission to cover all geotechnical risks presented in this document, as well as qualifying and caveating their tender submission accordingly.	Unlikely (Serious)	Moderate
2	Weak or compressible Superficial Deposits leading to excessive settlement, insufficient bearing capacity or slope instability	Earthworks, Foundations	Peat poses a potential risk to the scheme due to its inherent variability in composition, sporadic distribution and presence of deleterious material and obstructions. Extent and depth of peat and other soft superficial deposits are not well confirmed, leading to low and variable bearing capacities and/or high total	Likely (Serious)	Moderate	GI proposed spread over the whole site area with allowance for geotechnical testing. The encountered ground conditions should be reviewed and characteristic values of parameters should be determined where appropriate, based on in situ and laboratory test results. The encountered ground conditions should be used to inform proposed founding depths for foundations, and to inform proposed slope angles.	Unlikely (Serious)	Moderate



Risk Ref.	Risk	Works Affected	Details	Likelihood (Severity)	Risk Grading	Mitigation	Likelihood (Severity) After mitigation	Risk Grading After mitigation
			or differential settlement. Foundations may need to be deepened to reach more competent strata. Proportion of the superficial deposits will be unsuitable for reuse as upfill earthworks, increasing requirement for imported fill.					
3	Contaminated Soil	Excavations, Earthworks, Foundations	Ground investigation undertaken by SSE has widely spaced exploratory holes. Potential for contaminated soil due to current land use as Christmas tree farm (pesticides, machinery oil/fuel leaks). Delays to programme and costs associated with remediation, handling, off-site disposal and protective measures.	Likely (Moderate)	Moderate	Site specific Ground Investigation proposed with geochemical testing to determine extent and level of contamination. If required, specialists to be consulted to determine remedial works and safe systems of work.	Unlikely (Moderate)	Low
4	High / perched groundwater resulting in collapse of excavations, instability of slopes or reduced bearing capacity	Excavations, Earthworks, Foundations	Groundwater identified as being with 1 m of ground level in borehole installations, although this is not site wide where groundwater is generally deeper or absent within top 10 m. GI available for area adjacent to site suggests a low groundwater level but also a significant presence of peat.	Unlikely (Serious)	Moderate	The Contractor is to manage this risk during construction and should consider use of pre-treatment or management (pumping or other) of groundwater to manage the stability of excavations if required.	Unlikely (Serious)	Moderate
5	Chemical attack on buried concrete resulting in deterioration of foundations	Foundations	There is the potential for ground conditions that could give rise to chemical attack on buried concrete.	Unlikely (Serious)	Moderate	Samples of natural deposits have been tested in accordance with ICE Suite D, and the results should be assessed in accordance with BRE Special Digest 1. As part of detailed design, the specification of concrete should give due cognisance to the geotechnical chemistry test results from the proposed ground investigation and the resulting classifications in accordance with BRE Special Digest 1.	Unlikely (Serious)	Moderate
6	Buried obstructions or hard stratum resulting in construction delays	Excavations, Foundations	Tree stumps and roots and shallow rockhead all indicate the potential for obstructions to be encountered within the ground. Obstructions have the potential to cause delays and increases in construction cost if unforeseen, or if encountered at shallower depths than anticipated.	Extremely Likely (Moderate)	Moderate	The Contractor should review the data from the ground investigation to inform the selection of suitable method(s) of excavation.	Likely (Moderate)	Moderate
7	Unexploded ordnance	Excavations	The risk presented by unexploded ordnance should be considered for all sites.	Likely (Serious)	High	A UXO desk study and risk assessment has been undertaken which graded the site as low risk.	Unlikely (Serious)	Moderate



Dick (Gradina	Severity								
NISK (Risk Grading		Minor	Moderate	Serious	Catastrophic				
	Extremely Unlikely	Very Low	Very Low	Very Low	Low	Moderate				
	Unlikely	Very Low	Low	Low	Moderate	Moderate				
Likelihood	Likely	Very Low	Low	Moderate	Moderate	High				
	Extremely Likely	Low	Moderate	Moderate	High	High				
	Almost Certain	Low	Moderate	High	High	High				

The risk gradings are defined as follows:

- Very Low: very unlikely to affect the development
- Low: unlikely to affect development but cannot be entirely ruled out and contingency should be made for further assessment or mitigation during construction stage
- Moderate: likely to affect the development with moderate impact to programme or cost and further assessment required
- High: expected to affect the development with significant impact further assessment required during design and / or construction phase

The severities are defined as follows:

- Trivial: trivial damage or loss (no human injury)
- Minor: minor damage or loss (slight injury or illness)
- Moderate: moderate damage or loss (moderate injury or illness)
- Serious: substantial damage or loss (serious injury or illness)
- Catastrophic: catastrophic damage or loss (fatality or fatal injury)



Table C-2 – Geotechnical Assumptions Register

Assumption Ref.	Assumption	Works Affected	Details	Responsibility / Owner	Related Geotechnical Risks (See Table C-1)	Proposed Action
1	Rockhead level	Earthworks, Excavations, Foundations	Rockhead level has been modelled by exploratory hole information. Ground investigation data available for the site is widely spaced and therefore rockhead level had been interpolated between exploratory hole locations and therefore may not be locally accurate.	Contractor	1, 2, 3, 6	The tender design undertaken is based on the draft factual report for the 2023 ground investigation. Supplementary GI is proposed spread over the whole site area with allowance for geotechnical testing and will be used to confirm ground model and properties. Appropriate cost and contingency allowances should be made by the Contractor in their tender submission to cover the risk of significant variation from the assumed rockhead level, in addition to caveating/qualifying their tender accordingly, to reflect the limited nature of the information this presumed rockhead level is based on.
2	Presumed properties of superficial deposits	Earthworks, Foundations	It is assumed that for certain lightly loaded structures which are not sensitive to settlement, that the natural superficial deposits will provide sufficient bearing capacity for suitably designed shallow foundations. It is assumed that Peat will be excavated and replaced with granular fill due to compressibility. Extent, depth and composition of peat at site location not well defined.	Contractor	1, 2, 3	Supplementary GI is proposed spread over the whole site area with allowance for geotechnical testing and will be used to confirm ground model and properties. Appropriate cost and contingency allowances should be made by the Contractor in their tender submission to cover the risk of significant variation of material properties on a localised basis, in addition to caveating/qualifying their tender accordingly. This should include consideration of additional costs related to the foundations of lightly loaded structures needing to be deepened to bear onto bedrock, due to superficial deposits being weaker and/or more compressible than assumed.
3	Reuse of site won fill	Earthworks	It is currently assumed that 50% of the site won superficial deposits (excluding Peat) and rock can be reused for upfilling earthworks as engineered fill. Site won fill originating from superficial deposits may require screening and/or modification (e.g., lime, cement).	Contractor	1, 2, 3	Supplementary GI is proposed spread over the whole site area with allowance for geotechnical testing and will be used to confirm ground model and properties. Appropriate cost and contingency allowances should be made by the Contractor in their tender submission to cover the risk of significant variation of material properties on a localised basis, in addition to caveating/qualifying their tender accordingly.
4	Foundation type(s)	Foundations	It is assumed that conventional ground bearing shallow foundations are likely to be suitable at the site, bearing within either placed engineered granular fill or the existing Glacial Till, weathered rock or bedrock. The allowable bearing capacity of placed granular fill or rock is anticipated to be >150 kN/m², which is the minimum requirement for the sub formation as specified within SSEN Specification for Earthworks (SP-NET-CIV-501, rev 2.0)	Contractor	1 to 6	Supplementary GI is proposed spread over the whole site area with allowance for geotechnical testing and will be used to confirm ground model and properties. Appropriate cost and contingency allowances should be made by the Contractor in their tender submission to cover the risk of significant variation of material properties on a localised basis, in addition to caveating/qualifying their tender accordingly. This should include consideration of additional costs related to the foundations of lightly loaded structures needing to be deepened to bear onto bedrock, due to superficial deposits being weaker and/or more compressible than assumed.